



**ROBERT M. ARNOLD PUBLIC HEALTH SCIENCES BUILDING
THE FRED HUTCHINSON CANCER RESEARCH CENTER
SEATTLE, WASHINGTON**

TECHNICAL REPORT 3

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EXECUTIVE SUMMARY

The Robert M. Arnold Public Health Sciences Building was constructed on the campus of the Fred Hutchinson Cancer Research Center (FHCRC). The Public Health Sciences Building houses four Programs: Epidemiology, Cancer Biology, Biostatistics & Mathematics, and Cancer Prevention. Both laboratories and offices occupy Arnold Building. The building height is five stories (60') above grade. The structure also extends three stories below ground. There is an entrance plaza, service road, and turnaround at the building entrance. These public spaces are supported by a portion of the submerged structure.

This report is an investigation into the main lateral force resisting system of Robert M. Arnold Building on the Fred Hutchinson Cancer Research Center's campus in Seattle, Washington. The site of the building exposes it to high lateral loads of both wind and seismic nature. The report discusses methods of both computer modeling and manual calculation of the applied forces, their distribution through the building, and the effect this causes on the main lateral force resisting system. It was noted on the structural drawings that the owner wanted the building's structural design to be above the minimum standards dictated by the building code. The investigation found forces comparable to those listed on the structural drawings. The findings also noted that the drift ratio of the building was well below serviceability limitations of the American Society of Civil Engineers minimum design loads for buildings.

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BACKGROUND INFORMATION

The Robert M. Arnold Building was designed and completed prior to the City of Seattle's adoption of the International Building Code (IBC). The applicable building code, when the building was designed, was the 1997 Uniform Building Code (UBC) as amended by the Department of Planning and Development. The design of concrete structures shall also be in accordance with standards set forth by the American Concrete Institution (ACI). The Seattle Building Code is comprised of the 1997 Uniform Building Code and the amendments made by the City of Seattle. The current building code in Seattle is now the IBC. These design requirements will also be examined. Further investigations, analyses, and designs will comply with the current code. It is therefore necessary to look at any differences between the design requirements set forth by design professionals, the UBC and the IBC.

The Uniform Building Code refers to the American Institute of Steel Construction (AISC) for design provisions of steel structures. Regarding concrete construction, the UBC has based its own provisions on the American Concrete Institute 318 but has not explicitly adopted the standard. Certain portions of the Uniform Building Code reference specific sections of the American Society of Civil Engineers (ASCE) 7. One specific example of this is wind design. The section of ASCE 7 on wind design is referenced. However the UBC specifies its own method for determining wind pressures.

The International Building Code refers to AISC's design provisions for steel construction. The IBC has also adopted ACI 318 for the design of concrete structures. ASCE 7 is referenced regarding the minimum load for buildings.

GRAVITY LOADS

Dead Loads

As specified by the Seattle Building Code, the dead loads are considered to be, “the weight of all materials and fixed equipment incorporated into the structure”. Unlike the live loads, there is no table specified in the code. Where necessary, minimum design dead loads from ASCE 7 will be used.

FLOOR DEAD LOADS

DESCRIPTION

SUPERIMPOSED

MECHANICAL & ELECTRICAL ALLOWANCE	5	LB/FT ²
PARTITION LOAD	20	LB/FT ²
FLOOR FINISHES	2.5	LB/FT ²
CEILING FINISHES	2.5	LB/FT ²
TOTAL	30	LB/FT²

NON-SUPERIMPOSED

CONCRETE	150	LB/FT ³
TOTAL	150	LB/FT³
COMPOSITE CONCRETE DECK	50	LB/FT ²
TOTAL	50	LB/FT²

TABLE 3-1

Live Loads

Table 3-2 shows the live loads as obtained from the code and also those obtained from the structural drawings. Certain loads are not specified by the Seattle Building Code and do not fall into a broader category. The loads listed on the structural drawings in some areas differ from the code. For the purpose of analysis, the live loads determined by the design professionals will be used. The structural engineers had more information regarding building occupancy, building equipment, and building use. The office live load

takes into account the additional loads of filing systems. In accordance with the Seattle Building Code, reduction of live loads is permitted. However, the structural engineers have specified that there will be no live load reduction for the first level through the fourth level.

LIVE LOADS

DESCRIPTION	UNIFORM LOAD (LB/FT ²)		
	UNIFORM BUILDING CODE	STRUCTURAL DRAWINGS	INTERNATIONAL BUILDING CODE
FLOOR			
OFFICES	50	80	50
LEVELS 1—4 (OFFICE)	50	75	50
LABORATORIES	-	100	60
INTERSTITIAL	-	25	-
CORRIDORS	100	100	100
PARKING	50	50	40
SIDEWALKS & DRIVEWAYS	250	250	250
ROOF			
ROOF	25	25	20

TABLE 3-2

DESCRIPTION OF STRUCTURAL SYSTEM

Arnold Building is an interesting collage of structural systems. Different portions of this building employ different methods of supporting the necessary loads. The building itself consists of five stories above grade plus a mechanical “penthouse” on the roof, while also extending 3 stories below grade. The triangular transfer of load around the atrium provides an element of structural complexity unseen in rectilinear buildings. Arnold Building houses the Public Health Science Department of the Fred Hutchinson Cancer Research Center. FHCRC specified that the building be designed to a standard of structural integrity higher than that of the code.

Foundation

The foundation of the Public Health Sciences Building consists mainly of spread footings and wall footings. Where the foundation is required to resist lateral loads carried down by shear walls, the building uses deeper drilled piers. The average footing is about 12 square feet, however, sizes ranging from eight feet square to 28 feet by 24 feet. The depth ranges from 30 inches to 48 inches deep, but is typically around 40 inches deep.

Framing

The framing of Arnold Building is mainly composed of concrete structural elements; however, there are some portions of the building where steel has been used. Steel framing was used for the stairs and skylight in the atrium. A special stipulation was made by the structural engineers that the structure of the atrium be designed such that it would not cause any torsional load on the rest of the building. The columns on the fifth story are made of tube steel with typical size being TS 12x12x5/8. Steel was also employed in the design of the roof structure that houses the building’s mechanical equipment. The typical steel column in this area is a TS 4x4x4 ¼. The irregularity of the steel roof structure lends itself to atypical beam and girder sizes. They range from W 10x12 to W 30x132. There also are a few steel columns in the main structure.

Almost all of the remaining portions of the structure are made of concrete. The columns are continuous cast in place reinforced concrete. The typical columns are 24 inches square and are on an average grid of 30 feet by 30 feet. The columns do not taper towards the top; however, the amount of reinforcement can vary. The shape of some columns varies. On certain floors, columns have a diameter of 24 inches instead of a width of 24 inches. Supporting Campus Drive, the turnaround, and the entrance plaza, under which the building extends, is an area of the building which uses cast in place reinforced concrete. The average beam is 24 inches wide by 30 inches deep.

Structural Slabs

The floor system of Arnold Building is mainly composed of two way post-tensioned concrete floor slabs. The slab in the basement is not post-tensioned but instead is made of fiber reinforced concrete. The portion of the building that is under the entrance plaza uses reinforced concrete slabs. The roof slab is composed of reinforced concrete. With the noted exceptions the typical floor system is a flat port-tensioned concrete slab with drop panels.

SEISMIC LOADING

Computer Modeling

The mass of building components plays a pivotal role when the site is subjected to seismic excitation. It is critical that these masses and their distribution throughout the building be determined accurately. In a structure as complex as the FHCRC's Public Health Sciences Building, manual calculation of mass properties becomes quite cumbersome.

A computer model in Bentley's RAM Structural System was generated for Arnold building. The model was used to determine the participating masses at each story. Some portions of structure that extend above the Upper Roof Level were lumped to the

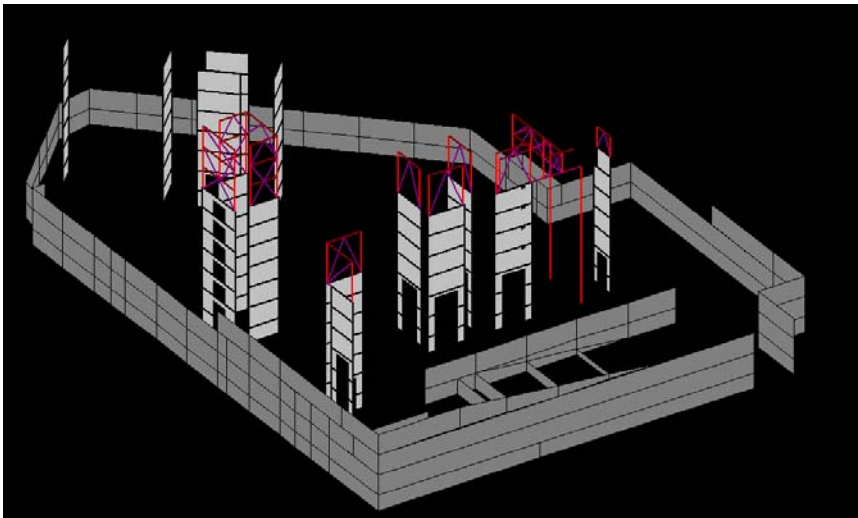


Figure 3-1

supporting roof level. These roof structures contain smaller braced frames that are discontinuous at the roof level. The roof level to which they were combined helps to distribute them to the main

lateral force resisting system. If they were modeled as frame members in RAM they would not be supported by other frame members that would result in various errors and warnings in the program. The roof level below these members is assumed to act as a rigid diaphragm and to distribute the loads to the braced frames which are part of the main lateral force resisting system. The method for calculating the seismic load in RAM Structural System was the Equivalent Lateral Force Procedure per ASCE 7-02. While the atrium provides a large opening in the slabs of the main floor levels it is within the limit

of 50% of the gross enclosed diaphragm area so that it does not constitute a Diaphragm Discontinuity Irregularity.

MANUAL VERIFICATION

Determination of Participating Mass

Manual methods of calculating weight and mass distribution were completed in order to verify the validity of the RAM model. The weights of the building components were first calculated. For steel members the linear weights as given in the AISC Steel Construction Manual were used. The linear nature of steel shapes simplifies locating the center of mass to locating the midpoint of the member. Steel construction constitutes only a small portion of the building, with the majority of Arnold Building being composed of concrete. The method for determining centers of mass for concrete elements was different. Having the structural plans drawn up in AutoCAD greatly simplified this task. The concrete structural elements of the building are mostly planar. Exploiting the planar geometry and Mass Properties command in AutoCAD concrete elements of the same depth and at the same elevation could be grouped together into regions; the area properties could be calculated. AutoCAD determines key properties such as area, location of the centroid, and moments of inertia. These areas could then be treated as plates. Using half of the story height for wall and column depths facilitates distributing the masses accurately by allowing half of the mass to be applied to the story below and half of the mass to be applied to the story above. Additional masses, such as exterior walls, elevator walls, and partition loads, were applied as either linear elements or area elements on the floor slab, similar to the application of mass dead loads in RAM and other computer modeling programs.

While determining the mass properties in AutoCAD of the various elements, the data was simultaneously entered into a Microsoft Access Database. The database allowed for the different elements of the building to be grouped according to story level. Querying the data allowed for the weighted coordinates of the center of mass for each story to be

determined, as well as the weight of the story. Additionally, an approximate mass moment of inertia could be determined for each story under the thin plate assumption.

Determination of Rigidities

Following the determination of masses and their distribution, rigidities of the main lateral force resisting system were determined. A simplified method of determining braced frame rigidities was used that only considers the contributions of the diagonal braces. The stiffness of these elements was calculated through another query in the Microsoft Access Database previously mentioned. Subsequently, the center of rigidity was calculated through a method of weighted coordinates.

The main portion of the lateral force resisting system is composed of shear walls. The rigidity for each shear wall was calculated at each floor level by applying a unit force at the top of the wall. The lateral deflection of the wall was determined based on shear and bending deformation of the wall. For shear walls with openings in them, initially the solid wall rigidity was determined. Subsequently the wall was then broken down into strips of pier and beam elements. The individual rigidities and deflections for these elements were determined and then built up to determine the overall rigidity of the wall. Calculations of each wall rigidity were carried out using Microsoft Excel spreadsheets. The variations of the amount of segments in each wall did not lend itself to the use of the database. Centers of rigidity for each story was determined by using weighted coordinates similar to the method used for the braced frames, however, this was carried out in spreadsheets. A few samples of the shear wall spreadsheet are included in the appendices, a complete set for all shear walls is available upon request. Also in the appendices is a summary of the rigidities for each level of each shear wall.

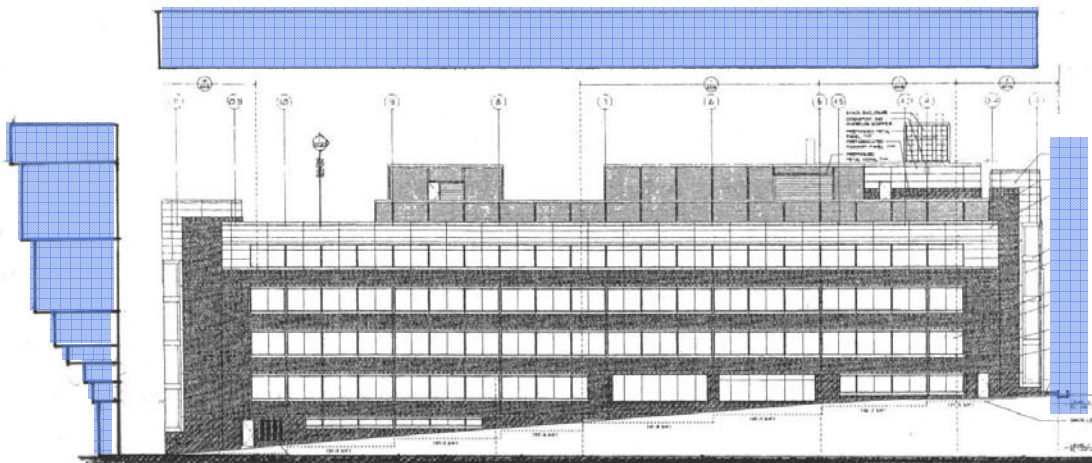
Seismic story forces were calculated using the Equivalent Lateral Force Procedure in accordance with ASCE-7. These calculations were completed using a spreadsheet which can be seen below. Through manual calculations the seismic base shear was determined to be 5938 kips, which is close to the 5980 kips listed on the structural drawings.

LEVEL	MANUAL	RAM
LVL PH	230.59	127.20
LVL RF	502.34	559.17
LVL ML	157.75	329.40
LVL 5	1612.01	1645.95
LVL 4	1086.70	1196.06
LVL 3	836.15	849.37
LVL 2	635.32	635.94
LVL 1	585.45	445.89
LVL D	291.86	131.49
BASE SHEAR	5938.16	5920.47

Table 3-3

Wind Loading

The design wind pressures for Robert M. Arnold building were determined in accordance with Method 2, the analytical method, of ASCE-7. This method was used in both the RAM Structural System, and manually. The manual calculation wind pressure was completed in Microsoft Excel. The pressures were then entered into the database aforementioned and forces were distributed to individual stories. The difference between the computer model and manual calculation of wind forces is due to limitations of software. The RAM model yields conservative results because Level 1 is only fully exposed on the east side of Arnold building due to changes in site grading. For examining the wind loads effects on the lateral system the loads determined in RAM will be used.



Distribution of Lateral Loads

The distribution of lateral story forces was based upon the relative rigidities of lateral force resisting elements. Both direct shear and torsional shear were distributed to the lateral members. Torsional shear was distributed based upon relative torsional rigidities. The appendices contain spreadsheets that calculate both the relative rigidities and the torsional rigidities/ torsional moment of inertia. In the appendices may be found the distribution of story shears to individual shear walls.

The lateral drift of Arnold Building was examined using the load cases generated in RAM structural system. The applied wind loads produced almost no drift at all. Seismic loads produced slightly higher story drifts but still were well within the 0.015 ratio provided by ASCE-7.

The shear walls of Robert M. Arnold Building typical call for 6000 psi concrete. In some locations it is noted on the shear wall elevations that an 8000 psi concrete mix is to be used for the lower stories. This is typically where the shear wall has an opening on the parking garage levels. All the shear walls have boundary elements and in some locations a special boundary element is required. These special boundary zones are discontinued at the slab of Level 3. Reinforcement in the special boundary zones is so extremely dense; mechanical couplers were required in order to reach full development without exceeding the maximum reinforcement ratio.

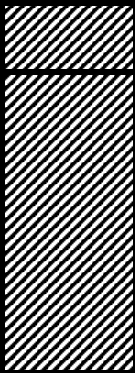
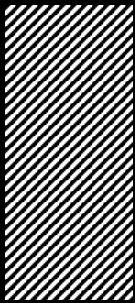
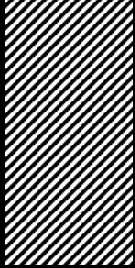
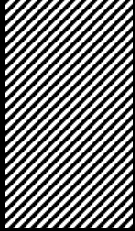
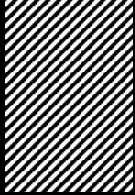
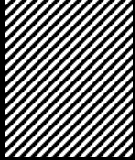




CONCLUSIONS

Using both a computer modeling program and hand calculations the seismic base shear of Arnold building was verified. Concerning lateral loads seismic loading seemed to be the controlling factor. The concrete construction of Arnold Building provides a significant amount of mass to participate in seismic events. The higher strength of the concrete in the shear combined, with the reinforcement of boundary elements and the use mechanical

couplers together have significant implications on construction costs. The investigation into the lateral system of Arnold building shows that the lateral force resisting system is more than sufficient for the applied loads.

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APPENDIX 1

MASS SUMMARY					
LEVEL	MASS		CENTER OF MASS		
			X	Y	Z
RF	547284		2607.69	2440.21	1249.28
UR	1380877		2572.75	2388.12	1121.77
LR	473049		2342.54	2034.85	1133.82
5	11566790		2281.66	1841.52	989.465
4	9715412		2253.39	1697.29	843.019
3	9712842		2253.28	1695.92	698.537
2	10199604		2223.6	1609.33	551.069
1	14358423		1981.94	2039.24	392.458
D	13242807		2013.23	1965.16	248.292

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APPENDIX 2

EQUIVALENT LATERAL FORCE PROCEDURE

Equivalent Lateral Force Procedure											
Site Class - B		T _L = 6									
S _s =	1.25	F _a =	1.0								
S ₁ =	0.50	F _v =	1.0								
S _{ms} =	1.25	SDS =	0.833								
S _{m1} =	0.50	SD1 =	0.333								
Importance Factor				Fundamental Period							
I =	1.0			h _n =	112						
Response Modification Factor [Table 12-2-1]				C _t =	0.02						
R =	6			x =	0.75						
Seismic Response Coefficient				T _a =	0.689						
C _s =	0.08068336										
				k =	1.377						
Level	w _i	h	h _x	w _i h _i ^k	C _{vx}	F _x	V _x	M _x			
LVL PH	547284	1095	112 ft	0.5 in	112.04	726934.80	0.038832	230.59	231	2613.33778	
LVL RF	1380877	2762	100 ft	8.5 in	100.71	1583643.10	0.084596	502.34	733	8555.04163	
LVL ML	473049	946	94 ft	6.5 in	94.54	497298.25	0.026565	157.75	891	29955.5633	
LVL 5	11566790	11567	83 ft	0.0 in	83.00	5081876.88	0.271466	1612.01	2503	83327.3404	
LVL 4	9715412	9715	70 ft	9.0 in	70.75	3425836.38	0.183003	1086.70	3589	180669.04	
LVL 3	9712842	9713	58 ft	6.0 in	58.50	2635977.28	0.140810	836.15	4426	332223.496	
LVL 2	10199604	10200	46 ft	3.0 in	46.25	2002862.16	0.106990	635.32	5061	545773.396	
LVL 1	14358423	14358	34 ft	0.0 in	34.00	1845633.05	0.098591	585.45	5646	828490.47	
LVL D	13242807	13243	21 ft	9.0 in	21.75	920087.76	0.049150	291.86	5938	1183949.98	
		0	9 ft	6.0 in	9.50	0.00	0.000000	0.00	5938	1498547.32	
		0	0 ft	6.0 in	0.50	0.00	0.000000	0.00	5938	1518994.03	
73598											
Seismic Base Shear											
V =	5938 kip										

APPENDIX 9

NORTH WIND

(NORTH Wind) Fred Hutchinson Cancer Research Center Wind Loading

International Building Code Loading Calculations
According to the methods specified in ASCE-07

Buildings of all Heights
 $p = qGCp - qh(Gcpi)$
 q : qz for windward wall at height z above ground
 qh for leeward wall, side walls and roof a mean roof height
 G : given in 6.6.1
 Cp given in Fig. 6-3
 $GCpi$ given in Table 6-4

$L = 382$ (parallel to wind)
 $V = 130$ mph
 $n_{mean} = 70.31$
 $B = 243$ (perpendicular to wind)
 Roof Type: Flat
 Roof Angle θ : 0
 Wind Direction: 90 (relative roof ridge)

Applicable Variables & Symbols [as defined my ASCE-07]

V basic wind speed
 K_d wind directionality factor
 I importance factor
 K_z velocity pressure exposure coefficient at height $z = h$
 K_h velocity pressure exposure coefficient at height z
 G gust effect factor
 $GCpi$ internal pressure coefficient

H Height of hill or escarpment relative to the upwind terrain, in feet
 L_h Distance upwind of crest to where the difference in ground elevation is half the height of the hill or escarpment, in feet
 K_1 Factor to account for shape of topographic feature and maximum speed-up effect
 K_2 Factor to account for reduction in speed-up with a distance upwind or downwind of crest
 K_3 Factor to account for reduction in speed-up with height above local terrain
 x Distance (upwind/downwind) from the crest to the building site, in feet
 z Height above local ground level, in feet
 μ Horizontal attenuation factor
 γ Height attenuation factor

Enclosure Classification: $GC_{pi} = +0.18/-0.18$

- Open Buildings
- Partially Enclosed Buildings
- Enclosed Buildings

Surface Roughness Category: C

- Urban & suburban areas, wooded areas or other terrain with numerous closely spaced obstructions having the size of single-family dwellings or larger.
- Open terrain with scattered obstructions having heights generally less than 30 ft (9.1 m). This category includes flat open country, grasslands, and all water surfaces in hurricane prone regions.
- Flat, unobstructed areas and water surfaces outside hurricane prone regions. This category includes smooth mud flats, salt flats, and unbroken ice.

Wind Directionality: $K_d = 0.85$

ONLY use wind directionality coefficient when the load combinations from ASCE 7-05 Section 2.3.2.4 used.

STRUCTURE TYPE

Buildings

- Main Wind Force Resisting System Components & Cladding
- Arched Roofs

Chimneys, Tanks, and Similar Structures

- Square
- Hexagonal
- Round
- Solid Signs

- Open Signs & Lattice Framework

Trussed Towers

- Triangular, square, rectangular
- All other cross sections

Building Exposure Category: C

Surface Roughness is Constant in upwind direction?

- yes no

Neither Exposure B nor Exposure D apply

- true false

- The hill, ridge, or escarpment is isolated and unobstructed upwind by other similar topographic features of comparable height for 100 times the height of the topographic feature (100H) or 2 mi (3.22 km), whichever is less. This distance shall be measured horizontally from the point at which the height H of the hill, ridge, or escarpment is determined.
- The hill, ridge, or escarpment protrudes above the height of upwind terrain feature within a 2-mi (3.22 km) radius in any quadrant by a factor of two or more.
- The structure is located as shown in Fig. 6-4 in the upper one-half of a hill or ridge or near the crest of an escarpment.
- $H/L_h \geq 0.2$.
- H is greater than or equal to 15 ft (4.5 m) for Exposures C and D and 60 ft (18 m) for Exposure B.

Importance Factor: $I = 1$

Hurricane prone region? 1

yes no

Occupancy Category? 2

I II III IV

Velocity Pressure Exposure Coefficients, K_h & K_z

Components & Cladding
 Main Wind Force Resisting System
 Low rise building designed using
 ASCE 7 Figure 6-10? yes no

Case 2

K_z or $K_h = 0.85$

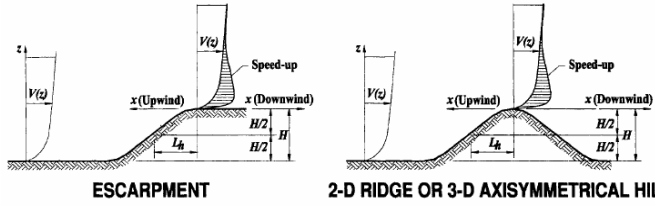
z	
h_{mean}	1.175157214
0	0.848884152
15	0.848884152
25	0.945264669
26.15	0.954257011
30	0.982252543
33	1.0021608
40	1.043580736
43	1.059591247
50	1.093775426
60	1.136574473
62	1.144447526
63	1.14830909
64	1.152122562
70	1.174064506
72.9	1.184141031
80	1.207537953
90	1.237854904
91.9	1.243311204
93.9	1.248959298
100	1.2656187

Gust Effect Factor

$G = 0.8867244$

Velocity Pressure (at height z)

$$q_z = 0.00256K_z K_{zt} K_d V^2$$



Topographical Data: $K_{rt} = 1.00$

Type of topography: Escarp

Escarpment 2-D Ridge 3-D Axisymmetrical Hill

Crest Location: Upwind

$H = 0$

$x = 0$

$L_h = 0$

$z = 0$

$K_1 = \text{\#VALUE!}$

$K_2 = \text{\#VALUE!}$

$K_3 = \text{\#VALUE!}$

External Pressure Coefficients

Walls

Windward	0.8
Leeward	-0.5
Side	-0.7

Roof

Perpendicular

Windward	N/A
Leeward	N/A

Parallel

0 to $h/2$	-0.9	-0.18
$h/2$ to h	-0.9	-0.18
h to $2h$	-0.5	-0.18
$>2h$	-0.3	-0.18

Design Wind Pressure

$$p = q G C_p - q_i (G C_{pi})$$

APPENDIX 9

NORTH WIND

Negative Internal pressure												
z h _{mean}	q 43.2157014	Windward	Leeward	Side	Leeward-Side-Roof							
					0 to h/2	h/2 to h	h to 2h	>2h	0 to h/2	h/2 to h	h to 2h	>2h
0	31.2172054	29.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
15	31.2172054	29.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
25	34.761541	32.44	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
26.15	35.092229	32.67	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
30	36.1217479	33.40	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
33	36.8538621	33.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
40	38.3770554	35.00	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
43	38.9658323	35.42	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
50	40.222935	36.31	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
60	41.7968443	37.43	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
62	42.0863711	37.63	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
63	42.2283778	37.73	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
64	42.3686159	37.83	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
70	43.1755178	38.41	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
72.9	43.5460758	38.67	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
80	44.4064837	39.28	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
90	45.5213714	40.07	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
91.9	45.7220235	40.21	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
93.9	45.9297288	40.36	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
100	46.5423699	40.80	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80

Positive Internal Pressure											
z h _{mean}	Windward	Leeward	Side	Leeward-Side-Roof							
				0 to h/2	h/2 to h	h to 2h	>2h	0 to h/2	h/2 to h	h to 2h	>2h
0	-32.69	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
15	-32.69	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
25	-35.52	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
26.15	-35.78	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
30	-36.61	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
33	-37.19	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
40	-38.41	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
43	-38.88	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
50	-39.88	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
60	-41.13	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
62	-41.37	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
63	-41.48	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
64	-41.59	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
70	-42.24	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
72.9	-42.53	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
80	-43.22	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
90	-44.11	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
91.9	-44.27	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
93.9	-44.43	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
100	-44.92	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76

TECHNICAL REPORT 3

APPENDIX 9

NORTH WIND

APPENDIX 9

WEST WIND

(West Wind) Fred Hutchinson Cancer Research Center Wind Loading

International Building Code Loading Calculations
According to the methods specified in ASCE-07

Buildings of all Heights
 $p = qGCp - qh(Gcpi)$
 q : qz for windward wall at height z above ground
 qh for leeward wall, side walls and roof a mean roof height
 G : given in 6.6.1
 Cp given in Fig. 6-3
 $GCpi$ given in Table 6-4

$L = 382$ (parallel to wind)
 $V = 130$ mph
 $B = 243$ (perpendicular to wind)
 $n_{mean} = 70.31$
 Roof Type: Flat
 Roof Angle θ : 0
 Wind Direction: 90 (relative roof ridge)

H Height of hill or escarpment relative to the upwind terrain, in feet
 L_h Distance upwind of crest to where the difference in ground elevation is half the height of the hill or escarpment, in feet
 K_1 Factor to account for shape of topographic feature and maximum speed-up effect
 K_2 Factor to account for reduction in speed-up with a distance upwind or downwind of crest
 K_3 Factor to account for reduction in speed-up with height above local terrain
 x Distance (upwind/downwind) from the crest to the building site, in feet
 z Height above local ground level, in feet
 μ Horizontal attenuation factor
 γ Height attenuation factor

Applicable Variables & Symbols [as defined by ASCE-07]

V basic wind speed
 K_d wind directionality factor
 I importance factor
 K_z velocity pressure exposure coefficient at height $z = h$
 K_h velocity pressure exposure coefficient at height z
 G gust effect factor
 $GCpi$ internal pressure coefficient

Enclosure Classification: $GC_{pi} = +0.18/-0.18$

- Open Buildings
- Partially Enclosed Buildings
- Enclosed Buildings

Surface Roughness Category: C

- Urban & suburban areas, wooded areas or other terrain with numerous closely spaced obstructions having the size of single-family dwellings or larger.
- Open terrain with scattered obstructions having heights generally less than 30 ft (9.1 m). This category includes flat open country, grasslands, and all water surfaces in hurricane prone regions.
- Flat, unobstructed areas and water surfaces outside hurricane prone regions. This category includes smooth mud flats, salt flats, and unbroken ice.

Wind Directionality: $K_d = 0.85$

ONLY use wind directionality coefficient when the load combinations from ASCE 7-05 Section 2.3.2.4 used.

STRUCTURE TYPE

- Buildings
 - Main Wind Force Resisting System
 - Components & Cladding
- Arched Roofs
- Chimneys, Tanks, and Similar Structures
 - Square
 - Hexagonal
 - Round
- Solid Signs
- Open Signs & Lattice Framework
- Trussed Towers
 - Triangular, square, rectangular
 - All other cross sections

Building Exposure Category: C

Surface Roughness is Constant in upwind direction?

yes no

Neither Exposure B nor Exposure D apply

true false

- The hill, ridge, or escarpment is isolated and unobstructed upwind by other similar topographic features of comparable height for 100 times the height of the topographic feature (100H) or 2 mi (3.22 km), whichever is less. This distance shall be measured horizontally from the point at which the height H of the hill, ridge, or escarpment is determined.
- The hill, ridge, or escarpment protrudes above the height of upwind terrain features within a 2-mi (3.22 km) radius in any quadrant by a factor of two or more.
- The structure is located as shown in Fig. 6-4 in the upper one-half of a hill or ridge or near the crest of an escarpment.
- $H/L_h \geq 0.2$.
- H is greater than or equal to 15 ft (4.5 m) for Exposures C and D and 60 ft (18 m) for Exposure B.

Importance Factor: $I = 1$

Hurricane prone region? 1

yes no

Occupancy Category? 2

I II III IV

Velocity Pressure Exposure Coefficients, K_1 , & K_2

Components & Cladding
 Main Wind Force Resisting System
 Low rise building designed using ASCE 7 Figure 6-10? yes no

Case 2

K_1 or $K_2 = 0.85$

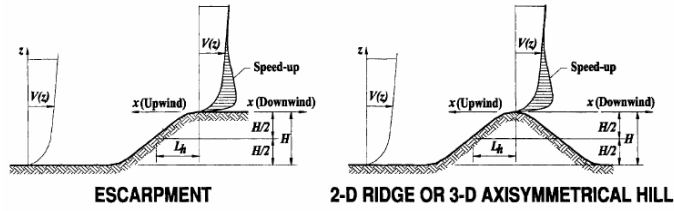
z	
1.75157214	
0	0.848884152
15	0.848884152
25	0.945264669
26.15	0.954257011
30	0.982252543
33	1.0021608
40	1.043580736
43	1.059591247
50	1.093775426
60	1.136574473
62	1.144447526
63	1.14830909
64	1.152122562
70	1.174064506
72.9	1.184141031
80	1.207537953
90	1.237854904
91.9	1.243311204
93.9	1.248959298
100	1.2656187

Gust Effect Factor

$G = 0.8867244$

Velocity Pressure (at height z)

$q_z = 0.00256K_1K_2K_3V^2I$



Topographical Data: $K_{zt} = 1.00$

Type of topography: Escarp

Escarpment 2-D Ridge 3-D Axisymmetrical Hill

Crest Location: Upwind

$H = 0$

$x = 0$

$L_h = 0$

$z = 0$

$K_1 = \text{\#VALUE!}$

$K_2 = \text{\#VALUE!}$

$K_3 = \text{\#VALUE!}$

External Pressure Coefficients

Walls

Windward	0.8
Leeward	-0.5
Side	-0.7

Roof

Perpendicular

Windward	N/A
Leeward	N/A

Parallel

0 to h/2	-0.9	-0.18
h/2 to h	-0.9	-0.18
h to 2h	-0.5	-0.18
>2h	-0.3	-0.18

Design Wind Pressure

$p = q GCp - qi(GCpi)$

APPENDIX 9

WEST WIND

Negative Internal pressure												
z h _{mean}	q 43.2157014	Leeward-Side-Roof										
		Windward	Leeward	Side	0 to h/2		h/2 to h		h to 2h		>2h	
0	31.2172054	29.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
15	31.2172054	29.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
25	34.761541	32.44	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
26.15	35.092229	32.67	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
30	36.1217479	33.40	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
33	36.8538621	33.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
40	38.3770554	35.00	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
43	38.9658323	35.42	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
50	40.222935	36.31	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
60	41.7968443	37.43	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
62	42.0863711	37.63	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
63	42.2283778	37.73	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
64	42.3686159	37.83	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
70	43.1755178	38.41	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
72.9	43.5460759	38.67	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
80	44.4064837	39.28	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
90	45.5213714	40.07	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
91.9	45.7220235	40.21	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
93.9	45.9297286	40.36	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
100	46.5423699	40.80	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80

Positive Internal Pressure											
z h _{mean}	Windward	Leeward	Side	Leeward-Side-Roof							
				0 to h/2		h/2 to h		h to 2h		>2h	
0	-32.69	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
15	-32.69	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
25	-35.52	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
26.15	-35.78	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
30	-36.61	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
33	-37.19	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
40	-38.41	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
43	-38.88	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
50	-39.88	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
60	-41.13	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
62	-41.37	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
63	-41.48	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
64	-41.59	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
70	-42.24	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
72.9	-42.53	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
80	-43.22	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
90	-44.11	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
91.9	-44.27	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
93.9	-44.43	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
100	-44.92	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76

APPENDIX 9

EAST WIND

(East Wind) Fred Hutchinson Cancer Research Center Wind Loading

International Building Code Loading Calculations
According to the methods specified in ASCE-07

Buildings of all Heights
 $p = qGCp - qh(Gcpi)$
 q : qz for windward wall at height z above ground
 qh for leeward wall, side walls and roof a mean roof height
 G : given in 6.6.1
 Cp given in Fig. 6-3
 $GCpi$ given in Table 6-4

$L = 382$ (parallel to wind)
 $V = 130$ mph
 $B = 243$ (perpendicular to wind)
 $n_{mean} = 70.31$
 Roof Type: Flat
 Roof Angle θ : 0
 Wind Direction: 90 (relative roof ridge)

H Height of hill or escarpment relative to the upwind terrain, in feet
 L_h Distance upwind of crest to where the difference in ground elevation is half the height of the hill or escarpment, in feet
 K_1 Factor to account for shape of topographic feature and maximum speed-up effect
 K_2 Factor to account for reduction in speed-up with a distance upwind or downwind of crest
 K_3 Factor to account for reduction in speed-up with height above local terrain
 x Distance (upwind/downwind) from the crest to the building site, in feet
 z Height above local ground level, in feet
 μ Horizontal attenuation factor
 γ Height attenuation factor

Applicable Variables & Symbols [as defined by ASCE-07]

V basic wind speed
 K_d wind directionality factor
 I importance factor
 K_z velocity pressure exposure coefficient at height $z = h$
 K_h velocity pressure exposure coefficient at height z
 G gust effect factor
 $GCpi$ internal pressure coefficient

Enclosure Classification: $GC_{pi} = +0.18/-0.18$

- Open Buildings
- Partially Enclosed Buildings
- Enclosed Buildings

Surface Roughness Category: C

Urban & suburban areas, wooded areas or other terrain with numerous closely spaced obstructions having the size of single-family dwellings or larger.

Open terrain with scattered obstructions having heights generally less than 30 ft (9.1 m). This category includes flat open country, grasslands, and all water surfaces in hurricane prone regions.

Flat, unobstructed areas and water surfaces outside hurricane prone regions. This category includes smooth mud flats, salt flats, and unbroken ice.

Wind Directionality: $K_d = 0.85$

ONLY use wind directionality coefficient when the load combinations from ASCE 7-05 Section 2.3.2.4 used.

STRUCTURE TYPE

- Buildings
 - Main Wind Force Resisting System
 - Components & Cladding
- Arched Roofs
- Chimneys, Tanks, and Similar Structures
 - Square
 - Hexagonal
 - Round
- Solid Signs
- Open Signs & Lattice Framework
- Trussed Towers
 - Triangular, square, rectangular
 - All other cross sections

Building Exposure Category: C

Surface Roughness is Constant in upwind direction?

yes no

Neither Exposure B nor Exposure D apply

true false

- The hill, ridge, or escarpment is isolated and unobstructed upwind by other similar topographic features of comparable height for 100 times the height of the topographic feature (100H) or 2 mi (3.22 km), whichever is less. This distance shall be measured horizontally from the point at which the height H of the hill, ridge, or escarpment is determined.
- The hill, ridge, or escarpment protrudes above the height of upwind terrain features within a 2-mi (3.22 km) radius in any quadrant by a factor of two or more.
- The structure is located as shown in Fig. 6-4 in the upper one-half of a hill or ridge or near the crest of an escarpment.
- $H/L_h \geq 0.2$.
- H is greater than or equal to 15 ft (4.5 m) for Exposures C and D and 60 ft (18 m) for Exposure B.

Importance Factor: $I = 1$

Hurricane prone region? 1

yes no

Occupancy Category? 2

I II III IV

Velocity Pressure Exposure Coefficients, K_1 , & K_2

Components & Cladding
 Main Wind Force Resisting System
 Low rise building designed using ASCE 7 Figure 6-10? yes no

Case 2

K_1 or $K_2 = 0.85$

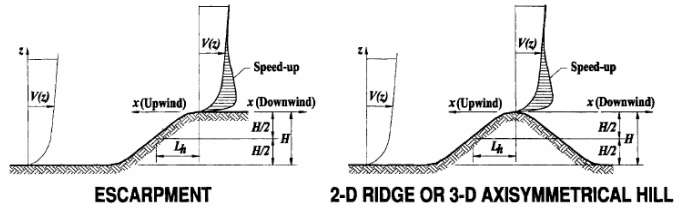
z	
1.75157214	
0	0.848884152
15	0.848884152
25	0.945264669
26.15	0.954257011
30	0.982252543
33	1.0021608
40	1.043580736
43	1.059591247
50	1.093775426
60	1.136574473
62	1.144447526
63	1.14830909
64	1.152122562
70	1.174064506
72.9	1.184141031
80	1.207537953
90	1.237854904
91.9	1.243311204
93.9	1.248959298
100	1.2656187

Gust Effect Factor

$G = 0.8867244$

Velocity Pressure (at height z)

$q_z = 0.00256K_1K_2K_3V^2$



Topographical Data: $K_{zt} = 1.00$

Type of topography: Escarp

Escarpment 2-D Ridge 3-D Axisymmetrical Hill

Crest Location: Upwind

$H = 0$ $x = 0$

$L_h = 0$ $z = 0$

$K_1 = \#VALUE!$ $K_2 = \#VALUE!$ $K_3 = \#VALUE!$

External Pressure Coefficients

Walls
 Windward 0.8
 Leeward -0.5
 Side -0.7

Roof

Perpendicular
 Windward N/A
 Leeward N/A
 Parallel
 0 to h/2 -0.9 -0.18
 h/2 to h -0.9 -0.18
 h to 2h -0.5 -0.18
 >2h -0.3 -0.18

Design Wind Pressure

$p = q GCp - qi(GCpi)$

APPENDIX 9

EAST WIND

Negative Internal pressure												
z h _{mean}	q 43.2157014	Leeward-Side-Roof										
		Windward	Leeward	Side	0 to h/2		h/2 to h		h to 2h		>2h	
0	31.2172054	29.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
15	31.2172054	29.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
25	34.761541	32.44	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
26.15	35.092229	32.67	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
30	36.1217479	33.40	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
33	36.8538621	33.92	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
40	38.3770554	35.00	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
43	38.9658323	35.42	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
50	40.222935	36.31	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
60	41.7968443	37.43	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
62	42.0863711	37.63	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
63	42.2283778	37.73	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
64	42.3686159	37.83	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
70	43.1755178	38.41	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
72.9	43.5460759	38.67	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
80	44.4064837	39.28	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
90	45.5213714	40.07	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
91.9	45.7220235	40.21	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
93.9	45.9297286	40.36	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80
100	46.5423699	40.80	-11.38	38.44	-26.71	0.88	-26.71	0.88	-11.38	0.88	-3.72	2.80

Positive Internal Pressure											
z h _{mean}	Windward	Leeward	Side	Leeward-Side-Roof							
				0 to h/2	h/2 to h		h to 2h		>2h		
0	-32.69	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
15	-32.69	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
25	-35.52	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
26.15	-35.78	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
30	-36.61	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
33	-37.19	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
40	-38.41	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
43	-38.88	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
50	-39.88	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
60	-41.13	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
62	-41.37	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
63	-41.48	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
64	-41.59	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
70	-42.24	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
72.9	-42.53	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
80	-43.22	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
90	-44.11	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
91.9	-44.27	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
93.9	-44.43	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76
100	-44.92	-26.94	22.88	-42.27	-14.68	-42.27	-14.68	-26.94	-14.68	-19.27	-12.76



Loads and Applied Forces

LOAD CASE: Seismic

Seismic ASCE 7-02 / IBC 2003 Equivalent Lateral Force
 Site Class: D Importance Factor: 1.00 Ss: 1.250 g S1: 0.500 g
 Fa: 1.000 Fv: 1.500 SDs: 0.833 g SD1: 0.500 g
 Seismic Use Group: III Seismic Design Category: D
 Provisions for: Force
 Ground Level: Floor_E

Dir	Eccent	R	Ta Equation	Building Period-T
X	None	6.0	Std,Ct=0.030,x=0.75	Calculated
Y	None	6.0	Std,Ct=0.030,x=0.75	Calculated

Dir	Ta	Cu	T	T-used	Eq95521-1	Eq95521-2	Eq95521-3	k
X	1.088	1.400	2.215	1.523	0.139	0.055	0.0367	1.511
Y	1.088	1.400	2.009	1.523	0.139	0.055	0.0367	1.511

Total Building Weight (kips) = 108204.78

APPLIED DIAPHRAGM FORCES

Type: EQ_IBC03_X_NoE_F

Level	Diaph.#	Ht ft	Fx kips	Fy kips	X ft	Y ft
PentHous_Roof	1	120.00	59.45	0.00	248.01	153.02
PH	1	110.00	21.95	0.00	141.33	219.67
PH	2	110.00	0.00	0.00	110.39	187.64
PH	3	110.00	0.00	0.00	280.73	312.01
PH	4	110.00	0.00	0.00	268.03	138.75
PH	5	110.00	45.80	0.00	247.88	258.04
PH	6	110.00	0.00	0.00	195.89	280.19
Roof	1	98.00	559.17	0.00	212.27	234.96
Mech	1	85.75	329.40	0.00	204.21	209.41
Floor_5	1	73.50	1645.95	0.00	188.48	148.89
Floor_4	1	61.25	1196.06	0.00	189.48	141.65
Floor_3	1	49.00	849.37	0.00	189.32	141.54
Floor_2	1	36.75	635.94	0.00	188.14	154.15
Floor_1	1	24.50	446.89	0.00	164.13	180.03
Floor_1	2	24.50	0.00	0.00	269.96	342.32
Floor_D	1	12.25	131.49	0.00	168.22	160.40

APPLIED STORY FORCES

Type: EQ_IBC03_X_NoE_F

Level	Ht ft	Fx kips	Fy kips
PentHous_Roof	120.00	59.45	0.00
PH	110.00	67.75	0.00
Roof	98.00	559.17	0.00
Mech	85.75	329.40	0.00



Loads and Applied Forces

Floor_5	73.50	1645.95	0.00
Floor_4	61.25	1196.06	0.00
Floor_3	49.00	849.37	0.00
Floor_2	36.75	635.94	0.00
Floor_1	24.50	446.89	0.00
Floor_D	12.25	131.49	0.00
		5921.48	0.00

APPLIED DIAPHRAGM FORCES

Type: EQ_IBC03_Y_NoE_F

Level	Diaph.#	Ht ft	Fx kips	Fy kips	X ft	Y ft
PentHous_Roof	1	120.00	0.00	59.45	248.01	153.02
PH	1	110.00	0.00	21.95	141.33	219.67
PH	2	110.00	0.00	0.00	110.39	187.64
PH	3	110.00	0.00	0.00	280.73	312.01
PH	4	110.00	0.00	0.00	268.03	138.75
PH	5	110.00	0.00	45.80	247.88	258.04
PH	6	110.00	0.00	0.00	195.89	280.19
Roof	1	98.00	0.00	559.17	212.27	234.96
Mech	1	85.75	0.00	329.40	204.21	209.41
Floor_5	1	73.50	0.00	1645.95	188.48	148.89
Floor_4	1	61.25	0.00	1196.06	189.48	141.65
Floor_3	1	49.00	0.00	849.37	189.32	141.54
Floor_2	1	36.75	0.00	635.94	188.14	154.15
Floor_1	1	24.50	0.00	446.89	164.13	180.03
Floor_1	2	24.50	0.00	0.00	269.96	342.32
Floor_D	1	12.25	0.00	131.49	168.22	160.40

APPLIED STORY FORCES

Type: EQ_IBC03_Y_NoE_F

Level	Ht ft	Fx kips	Fy kips
PentHous_Roof	120.00	0.00	59.45
PH	110.00	0.00	67.75
Roof	98.00	0.00	559.17
Mech	85.75	0.00	329.40
Floor_5	73.50	0.00	1645.95
Floor_4	61.25	0.00	1196.06
Floor_3	49.00	0.00	849.37
Floor_2	36.75	0.00	635.94
Floor_1	24.50	0.00	446.89
Floor_D	12.25	0.00	131.49
		0.00	5921.48



Loads and Applied Forces

RAM Frame v11.0
Jonathan Williams
DataBase: FHCRC_Existing

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Level Floor_1 has no Diaphragm. Story Force is NEGLECTED.
Level PH has no Diaphragm. Story Force is NEGLECTED.



LOAD CASE: Wind

Wind ASCE 7-02/IBC2003
 Exposure: B
 Basic Wind Speed (mph): 130.0 Importance Factor: 1.000
 Apply Directionality Factor, Kd = 0.85
 Use Topography Factor, Kzt: 1.00
 Use Calculated Frequency for X-Dir.
 Use Calculated Frequency for Y-Dir.
 Gust Factor for Flexible Structures, G: Use Calculated G for X-Dir.
 Gust Factor for Flexible Structures, G: Use Calculated G for Y-Dir.
 Damping Ratio for Flexible Structures= 0.01
 Mean Roof Height (ft): Top Story Height = 120.00
 Ground Level: Floor_E

WIND PRESSURES:

X-Direction: Natural Frequency = 0.452 Structure is Flexible
 Y-Direction: Natural Frequency = 0.498 Structure is Flexible
 CpWindward = 0.80 qLeeward (qh) = 38.28 psf
 GCpn (Parapet): Windward = 1.80 Leeward = -1.10

Height ft	Kz	Kzt	qz psf	Gust Factor G		CpLeeward		Pressure (psf)	
				X	Y	X	Y	X	Y
120.00	1.041	1.000	38.285	1.311	1.272	-0.500	-0.479	65.563	61.989
110.00	1.016	1.000	37.345	0.988	0.958	-0.498	-0.500	48.316	46.968
110.00	1.016	1.000	37.345	0.988	0.958	-0.498	-0.500	48.316	46.968
110.00	1.016	1.000	37.345	0.988	0.958	-0.498	-0.500	48.316	46.968
110.00	1.016	1.000	37.345	0.988	0.958	-0.498	-0.500	48.316	46.968
110.00	1.016	1.000	37.345	0.988	0.958	-0.498	-0.500	48.316	46.968
110.00	1.016	1.000	37.345	0.988	0.958	-0.498	-0.500	48.316	46.968
98.00	0.983	1.000	36.132	0.991	0.964	-0.500	-0.498	47.629	46.250
85.75	0.946	1.000	34.780	0.947	0.925	-0.500	-0.497	44.461	43.337
73.50	0.905	1.000	33.281	0.890	0.925	-0.500	-0.384	40.803	38.173
61.25	0.859	1.000	31.592	0.890	0.925	-0.500	-0.384	39.598	36.925
49.00	0.806	1.000	29.641	0.890	0.925	-0.500	-0.384	38.206	35.483
36.75	0.742	1.000	27.302	0.880	0.925	-0.500	-0.356	36.121	32.734
24.50	0.661	1.000	24.315	0.880	0.902	-0.500	-0.416	33.988	31.883
24.50	0.661	1.000	24.315	0.880	0.902	-0.500	-0.416	33.988	31.883
12.25	0.575	1.000	21.135	0.877	0.902	-0.500	-0.409	31.642	29.335
0.00	0.575	1.000	21.135	0.877	0.902	-0.500	-0.409	31.642	29.335

APPLIED DIAPHRAGM FORCES

Type: Wind_IBC03_1_X

Level	Diaph.#	Ht ft	Fx kips	Fy kips	X ft	Y ft
PentHous_Roof	1	120.00	17.78	0.00	248.00	154.69
PH	1	110.00	5.09	0.00	141.33	219.67
PH	2	110.00	6.26	0.00	110.40	187.62
PH	3	110.00	3.87	0.00	280.75	312.21



PH	4	110.00	6.03	0.00	267.21	139.14
PH	5	110.00	7.09	0.00	247.88	258.02
PH	6	110.00	15.33	0.00	195.88	275.38
Roof	1	98.00	98.67	0.00	194.33	223.50
Mech	1	85.75	117.51	0.00	171.75	208.96
Floor_5	1	73.50	157.33	0.00	171.75	155.65
Floor_4	1	61.25	184.69	0.00	171.75	128.50
Floor_3	1	49.00	177.80	0.00	171.75	128.50
Floor_2	1	36.75	176.17	0.00	171.54	137.48
Floor_1	1	24.50	173.06	0.00	145.75	146.03
Floor_1	2	24.50	10.74	0.00	267.75	341.17
Floor_D	1	12.25	165.38	0.00	145.75	143.32

APPLIED STORY FORCES

Type: Wind_IBC03_1_X

Level	Ht ft	Fx kips	Fy kips
PentHous_Roof	120.00	17.78	0.00
PH	110.00	43.66	0.00
Roof	98.00	98.67	0.00
Mech	85.75	117.51	0.00
Floor_5	73.50	157.33	0.00
Floor_4	61.25	184.69	0.00
Floor_3	49.00	177.80	0.00
Floor_2	36.75	176.17	0.00
Floor_1	24.50	183.81	0.00
Floor_D	12.25	165.38	0.00
		1322.81	0.00

APPLIED DIAPHRAGM FORCES

Type: Wind_IBC03_1_Y

Level	Diaph.#	Ht ft	Fx kips	Fy kips	X ft	Y ft
PentHous_Roof	1	120.00	0.00	13.62	245.67	153.02
PH	1	110.00	0.00	2.87	141.33	219.67
PH	2	110.00	0.00	3.00	110.40	187.62
PH	3	110.00	0.00	8.61	282.64	312.02
PH	4	110.00	0.00	12.15	264.62	139.17
PH	5	110.00	0.00	7.90	247.88	258.02
PH	6	110.00	0.00	10.42	195.88	275.38
Roof	1	98.00	0.00	79.64	200.72	227.83
Mech	1	85.75	0.00	114.40	185.91	196.27
Floor_5	1	73.50	0.00	114.72	171.75	128.50
Floor_4	1	61.25	0.00	109.14	171.75	128.50
Floor_3	1	49.00	0.00	104.33	171.75	128.50



Loads and Applied Forces

RAM Frame v11.0
Jonathan Williams
DataBase: FHCRC_Existing

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Floor_2	1	36.75	0.00	97.86	171.65	146.03
Floor_1	1	24.50	0.00	103.45	157.63	146.03
Floor_1	2	24.50	0.00	19.16	267.75	341.17
Floor_D	1	12.25	0.00	107.00	145.75	140.61

APPLIED STORY FORCES

Type: Wind_IBC03_1_Y

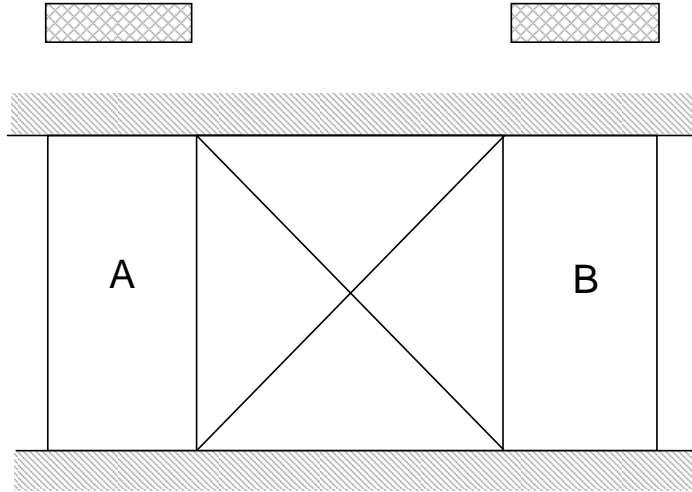
Level	Ht ft	Fx kips	Fy kips
PentHous_Roof	120.00	0.00	13.62
PH	110.00	0.00	44.95
Roof	98.00	0.00	79.64
Mech	85.75	0.00	114.40
Floor_5	73.50	0.00	114.72
Floor_4	61.25	0.00	109.14
Floor_3	49.00	0.00	104.33
Floor_2	36.75	0.00	97.86
Floor_1	24.50	0.00	122.61
Floor_D	12.25	0.00	107.00
		0.00	908.27

Level Floor_1 has no Diaphragm. Story Force is NEGLECTED.

Level PH has no Diaphragm. Story Force is NEGLECTED.

Wall Rigidity

WALL (OR PORTION THEREOF) GEOMETRY



$$R = \frac{1}{\Delta}$$

$$R_t = R_A + R_B$$

$h = \frac{99.5}{20}$
 $b = \frac{20}{48}$
 $da = \frac{48}{48}$
 $db = \frac{48}{48}$

END FIXITY: FIXED

$P = 1$ lbs
 $E_m = 5098235$ psi
 $E_r = 2124265$ psi

LONGITUDINAL

$R_A = 6853715.192$
 $R_B = 6853715.192$
 $R_T = 13707430.38$

TRANSVERSE

$R_A = 1780235$
 $R_B = 1780235$
 $R_T = 3560470$

Wall Rigidity (A)

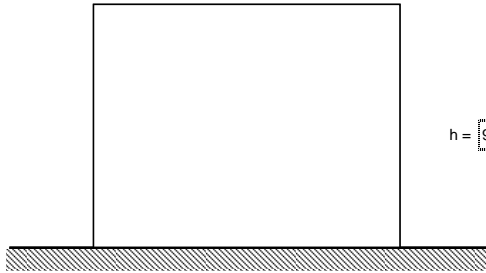
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction



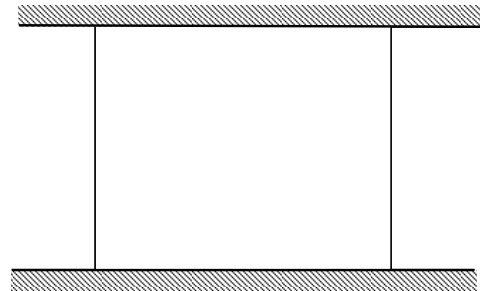
b = 20 INCHES

d = 48 INCHES



h = 99.5

CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED	
I = 184320 in ⁴	P = 1 lbs
h = 99.5 in	Em = 5098235 psi
d = 48 in	Er = 2124264.6 psi
A = 960 in ²	

$\Delta_m =$	8.73566E-08	$\Delta_t =$	1.45906E-07
$\Delta_v =$	5.85497E-08	R =	6853715.192

Wall Rigidity (B)

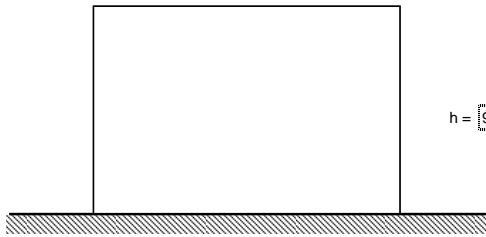
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction

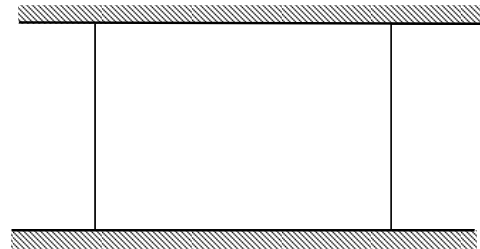


d = 48 INCHES

b = 20 INCHES



CANTILEVER



FIXED

h = 99.5

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED

I = 184320 in⁴
 h = 99.5 in
 d = 48 in
 A = 960 in²

P = 1 lbs
 E_m = 5098235 psi
 E_r = 2124264.6 psi

$\Delta_m = 8.73566\text{E-}08$

$\Delta_t = 1.45906\text{E-}07$

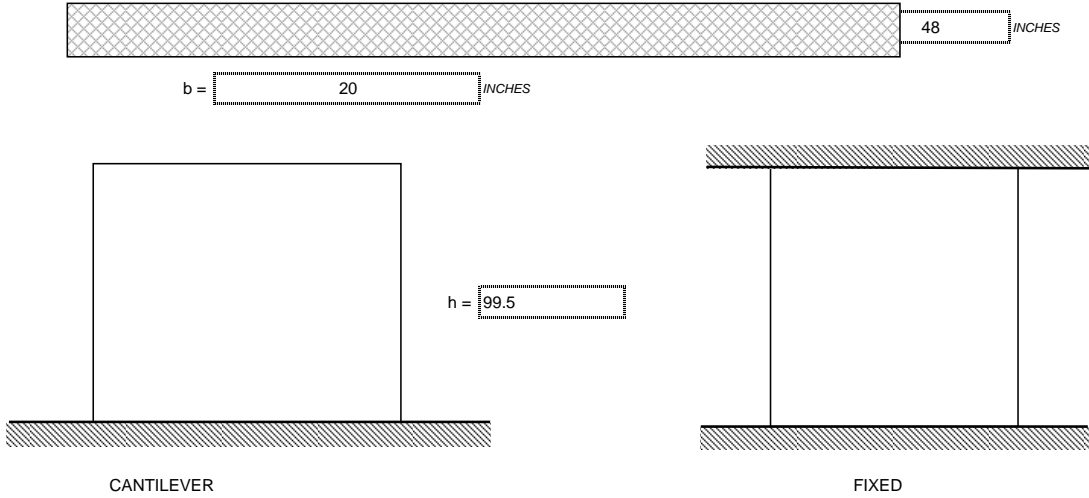
$\Delta_v = 5.85497\text{E-}08$

R = 6853715.192

Wall Rigidity (A)

WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction



$$R = \frac{1}{\Delta} \qquad R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m \qquad \Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I_m} = \frac{Ph^3}{3E_m I} \text{ (CANTILEVER)}$$

$$\Delta_v = \frac{1.2Ph}{E_v A} \qquad \Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} = \frac{Ph^3}{12E_m I} \text{ (FIXED) (FIXED)}$$

Input

END FIXITY: FIXED					
$I =$	32000	in ⁴	$P =$	1	lbs
$h =$	99.5	in	$E_m =$	5098235	psi
$d =$	20	in	$E_r =$	2124264.6	psi
$A =$	960	in ²			

$\Delta_m =$	5.03174E-07	$\Delta_t =$	5.61724E-07
$\Delta_v =$	5.85497E-08	$R =$	1780234.826

Wall Rigidity (B)

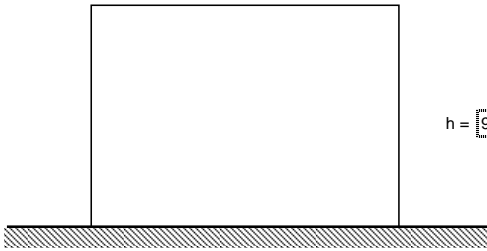
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction

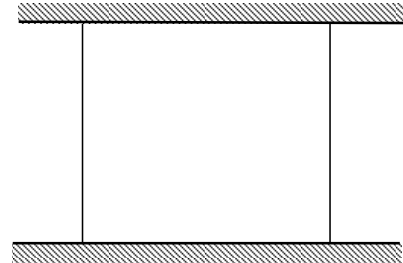


b = INCHES

d = INCHES



CANTILEVER



FIXED

h =

$$R = \frac{1}{\Delta}$$

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m \quad \Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I^m} = \frac{Ph^3}{3E_m I} \text{ (CANTILEVER)}$$

$$\Delta_v = \frac{1.2Ph}{E_v A} \quad \Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} = \frac{Ph^3}{12E_m I} \text{ (FIXED)}$$

Input

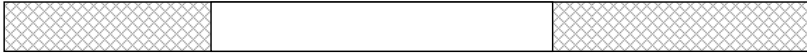
END FIXITY: FIXED			
l = <input type="text" value="32000"/>	in ⁴	P = <input type="text" value="1"/>	lbs
h = <input type="text" value="99.5"/>	in	Em = <input type="text" value="5098235"/>	psi
d = <input type="text" value="20"/>	in	Er = <input type="text" value="2124264.6"/>	psi
A = <input type="text" value="960"/>	in ²		

$\Delta_m =$	5.03174E-07	$\Delta_t =$	5.61724E-07
$\Delta_v =$	5.85497E-08	R =	1780234.826

Wall Rigidity (SOLID)

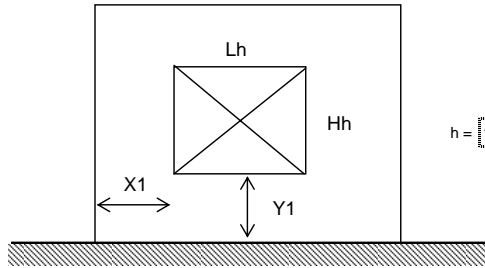
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction

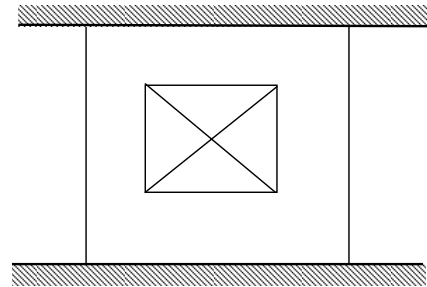


b = 20 INCHES

d = 204 INCHES



CANTILEVER



FIXED

h = 138.5

Hh = 123 INCHES
 Lh = 108 INCHES
 X1 = 48 INCHES
 Y1 = INCHES

END FIXITY: FIXED

Longitudinal Direction		
Element	Rigidity	Deflection
Solid Wall	44953372.4	2.22453E-08
Solid Midstrip	52138184.21	1.91798E-08
Left Pier	4212300.561	2.374E-07
Right Pier	4212300.561	2.374E-07
Effective Midstrip	8424601.123	1.187E-07
Effective Wall	8212510.007	1.21765E-07

P = 1 lbs
 Em = 5098234.989 psi
 Er = 2124264.579 psi

Transverse Direction		
Element	Rigidity	Deflection
Solid Wall	2954350.863	3.38484E-07
Solid Midstrip	4154835.329	2.40683E-07
Left Pier	977608.3128	1.0229E-06
Right Pier	4083955.605	2.44861E-07
Effective Midstrip	5061563.918	1.97567E-07
Effective Wall	3385609.384	2.95368E-07

Wall Rigidity (SOLID)

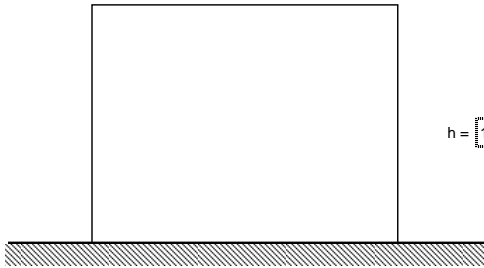
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction



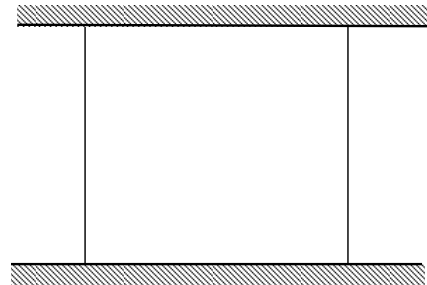
b = 20 INCHES

d = 204 INCHES



h = 138.5

CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

Input

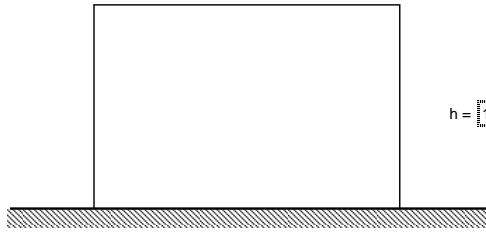
END FIXITY: FIXED	
l = 14149440 in4	P = 1 lbs
h = 138.5 in	Em = 5098234.99 psi
d = 204 in	Er = 2124264.58 psi
A = 4080 in2	

Δ_m =	3.06909E-09	Δ_t =	2.22453E-08
Δ_v =	1.91762E-08	R =	44953372.4

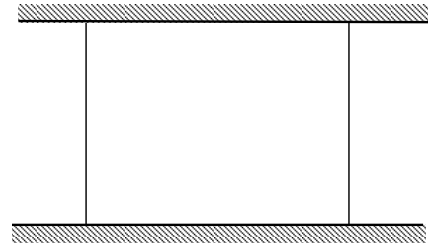
Wall Rigidity (SOLID MIDSTRIP)

WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction



CANTILEVER



FIXED



$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad \text{(CANTILEVER)}$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad \text{(FIXED)}$$

Input

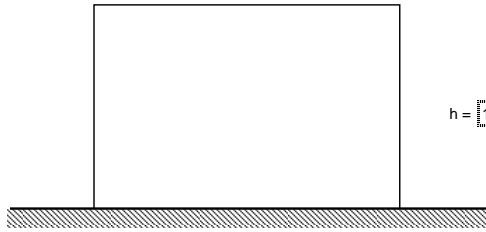
END FIXITY: FIXED			
I =	<input type="text" value="14149440"/>	in ⁴	
h =	<input type="text" value="123"/>	in	P = <input type="text" value="1"/> lbs
d =	<input type="text" value="204"/>	in	Em = <input type="text" value="5098234.99"/> psi
A =	<input type="text" value="4080"/>	in ²	Er = <input type="text" value="2124264.58"/> psi

$\Delta_m =$	2.14969E-09	$\Delta_t =$	1.91798E-08
$\Delta_v =$	1.70301E-08	R =	52138184.21

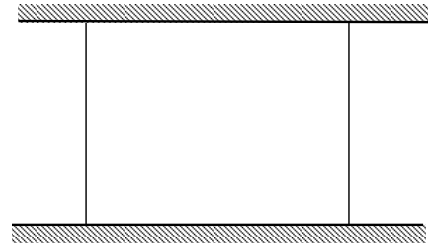
Wall Rigidity (LEFT PIER)

WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction



CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

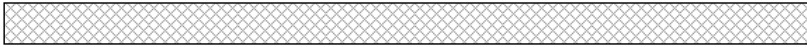
END FIXITY: FIXED			
I =	184320	in ⁴	
h =	123	in	
d =	48	in	
A =	960	in ²	
			P = 1 lbs
			Em = 5098234.99 psi
			Er = 2124264.58 psi

$\Delta_m =$	1.65022E-07	$\Delta_t =$	2.374E-07
$\Delta_v =$	7.2378E-08	R =	4212300.561

Wall Rigidity (RIGHT PIER)

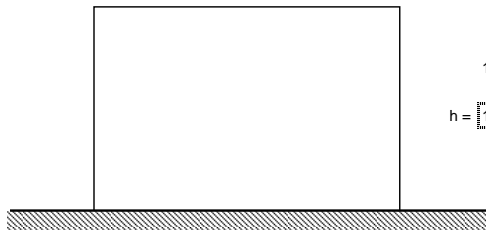
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction



b = 20 INCHES

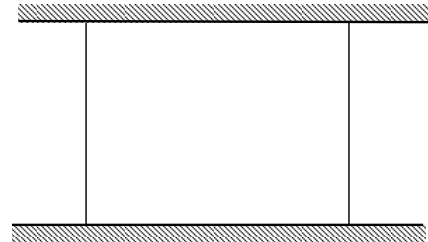
d = 48 INCHES



11'-6.5"

h = 123

CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED

I = 184320 in⁴
 h = 123 in
 d = 48 in
 A = 960 in²

P = 1 lbs
 E_m = 5098234.99 psi
 E_r = 2124264.58 psi

$\Delta_m = 1.65022\text{E-}07$

$\Delta_t = 2.374\text{E-}07$

$\Delta_v = 7.2378\text{E-}08$

R = 4212300.561

Wall Rigidity (SOLID)

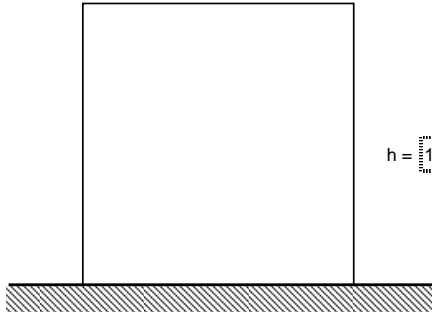
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction

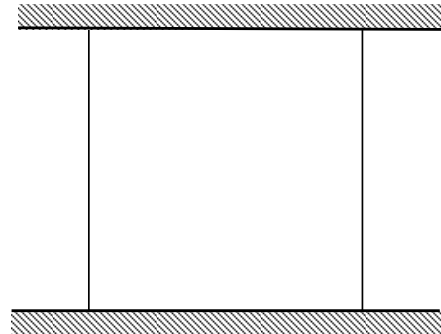


$d = 204$ INCHES

$b = 20$ INCHES



$h = 138.5$ INCHES



CANTILEVER

FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED	
$I = 136000$ in ⁴	$P = 1$ lbs
$h = 138.5$ in	$E_m = 5098234.99$ psi
$d = 20$ in	$E_r = 2124264.58$ psi
$A = 4080$ in ²	

$\Delta_m = 3.19308\text{E-}07$	$\Delta_t = 3.38484\text{E-}07$
$\Delta_v = 1.91762\text{E-}08$	$R = 2954350.863$

Wall Rigidity (SOLID MIDSTRIP)

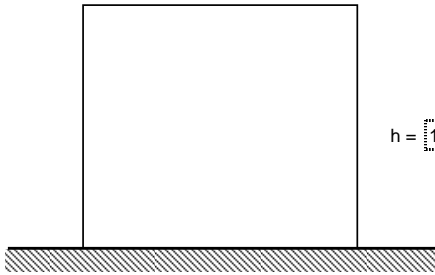
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction

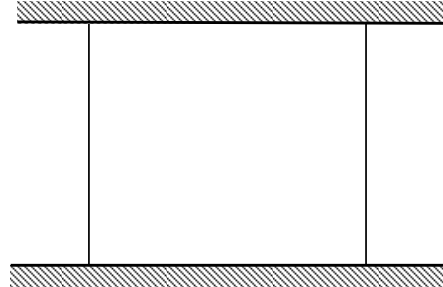


d = 204 INCHES

b = 20 INCHES



CANTILEVER



FIXED

h = 123

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED	
I = 136000 in ⁴	P = 1 lbs
h = 123 in	E _m = 5098234.99 psi
d = 20 in	E _r = 2124264.58 psi
A = 4080 in ²	

Δ_m =	2.23653E-07	Δ_t =	2.40683E-07
Δ_v =	1.70301E-08	R =	4154835.329

Wall Rigidity (LEFT PIER)

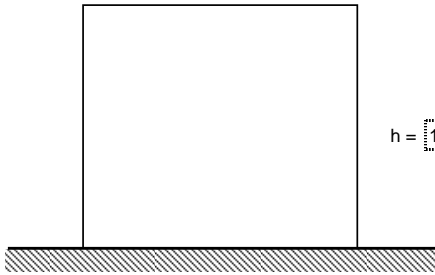
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction

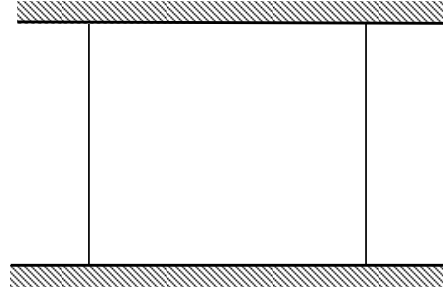


b = 20 INCHES

d = 48 INCHES



CANTILEVER



FIXED

h = 123

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED	
I = 32000 in ⁴	P = 1 lbs
h = 123 in	E _m = 5098234.99 psi
d = 20 in	E _r = 2124264.58 psi
A = 960 in ²	

$\Delta_m =$	9.50527E-07	$\Delta_t =$	1.0229E-06
$\Delta_v =$	7.2378E-08	R =	977608.3128

Wall Rigidity (RIGHT PIER)

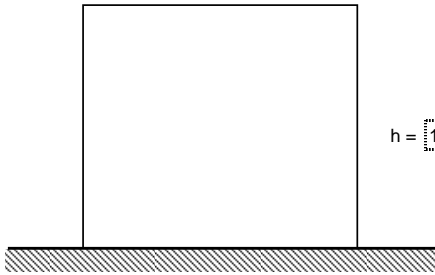
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction

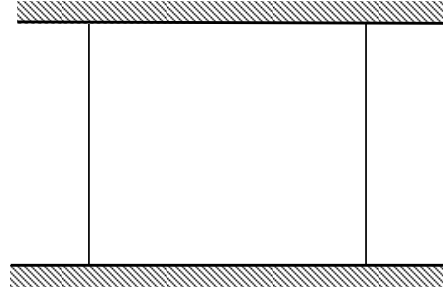


d = 282 INCHES

b = 20 INCHES



CANTILEVER



FIXED

h = 138.5

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED	
I = 188000 in ⁴	P = 1 lbs
h = 138.5 in	E _m = 5098234.99 psi
d = 20 in	E _r = 2124264.58 psi
A = 5640 in ²	

$\Delta_m =$	2.30989E-07	$\Delta_t =$	2.44861E-07
$\Delta_v =$	1.38721E-08	R =	4083955.605

Wall Rigidity

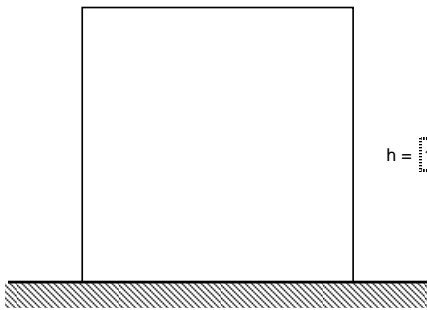
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction

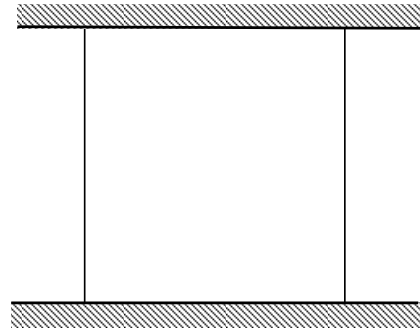


b = 20 INCHES

d = 204 INCHES



CANTILEVER



FIXED

h = 138.5

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED

I = 14149440 in⁴
 h = 138.5 in
 d = 204 in
 A = 4080 in²

P = 1 lbs
 E_m = 5098235 psi
 E_r = 2124265 psi

$\Delta_m = 3.06909\text{E-}09$

$\Delta_t = 2.22453\text{E-}08$

$\Delta_v = 1.91762\text{E-}08$

R = 44953372.4

Wall Rigidity

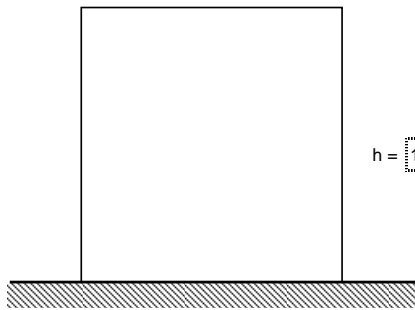
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction



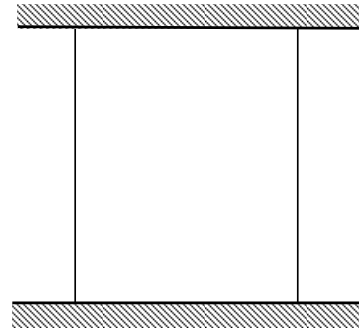
d = 204 INCHES

b = 20 INCHES



h = 138.5

CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED

I = 136000 in⁴
 h = 138.5 in
 d = 20 in
 A = 4080 in²

P = 1 lbs
 E_m = 5098235 psi
 E_r = 2124265 psi

$\Delta_m = 3.19308\text{E-}07$

$\Delta_t = 3.38\text{E-}07$

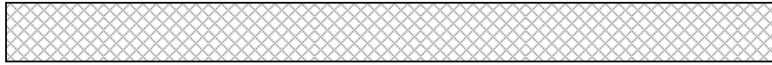
$\Delta_v = 1.91762\text{E-}08$

R = 2954351

Wall Rigidity

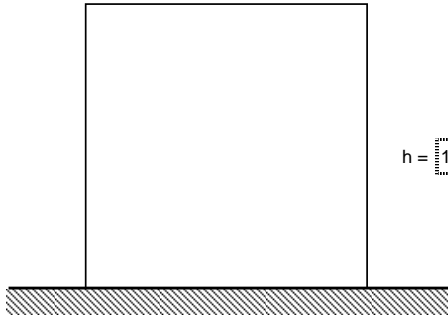
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction

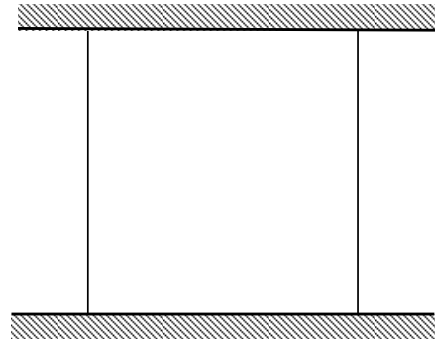


b = 20 INCHES

d = 204 INCHES



CANTILEVER



FIXED

h = 138.5

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

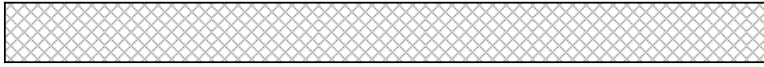
END FIXITY: FIXED	
I = 14149440 in ⁴	P = 1 lbs
h = 138.5 in	E _m = 5098235 psi
d = 204 in	E _r = 2124265 psi
A = 4080 in ²	

$\Delta_m =$	3.06909E-09	$\Delta_t =$	2.22453E-08
$\Delta_v =$	1.91762E-08	R =	44953372.4

Wall Rigidity

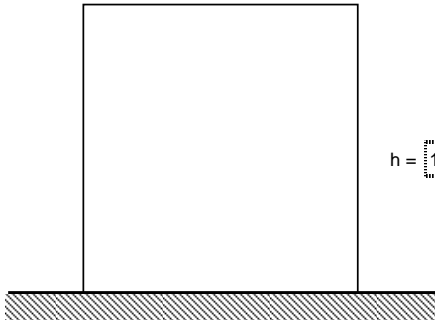
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction

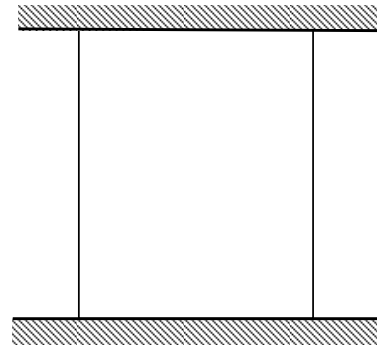


$d = 204$ INCHES

$b = 20$ INCHES



CANTILEVER



FIXED

$h = 138.5$

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED	
$I = 136000$ in ⁴	$P = 1$ lbs
$h = 138.5$ in	$E_m = 5098235$ psi
$d = 20$ in	$E_r = 2124265$ psi
$A = 4080$ in ²	

$\Delta_m = 3.19308\text{E-}07$	$\Delta_t = 3.38\text{E-}07$
$\Delta_v = 1.91762\text{E-}08$	$R = 2954351$

Wall Rigidity

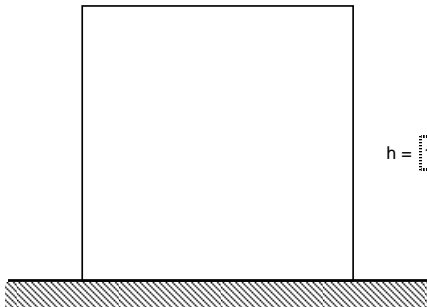
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction



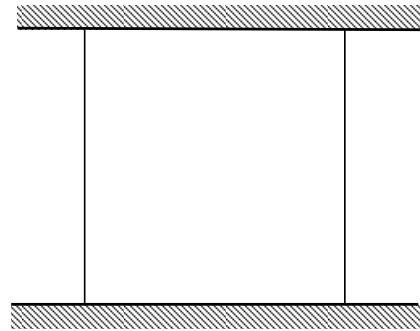
b = 20 INCHES

d = 204 INCHES



h = 138.5

CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED

l = 14149440 in⁴
 h = 138.5 in
 d = 204 in
 A = 4080 in²

P = 1 lbs
 E_m = 5098235 psi
 E_r = 2124265 psi

$\Delta_m = 3.06909\text{E-}09$

$\Delta_t = 2.22453\text{E-}08$

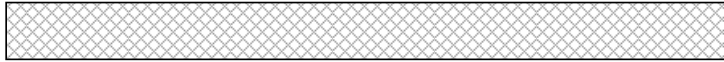
$\Delta_v = 1.91762\text{E-}08$

R = 44953372.4

Wall Rigidity

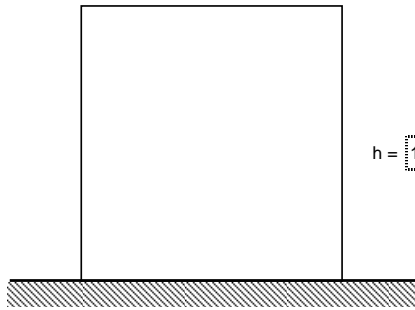
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction



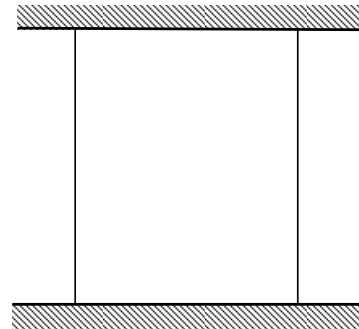
d = 204 INCHES

b = 20 INCHES



h = 138.5

CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED

l = 136000 in4
 h = 138.5 in
 d = 20 in
 A = 4080 in2

P = 1 lbs
 Em = 5098235 psi
 Er = 2124265 psi

$\Delta_m = 3.19308E-07$

$\Delta_t = 3.38E-07$

$\Delta_v = 1.91762E-08$

R = 2954351

Wall Rigidity

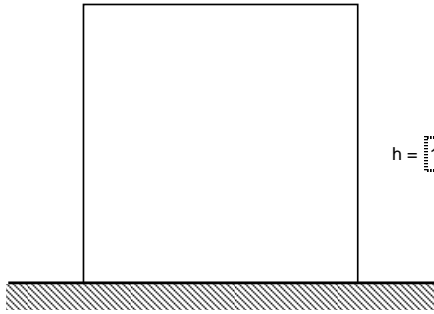
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction



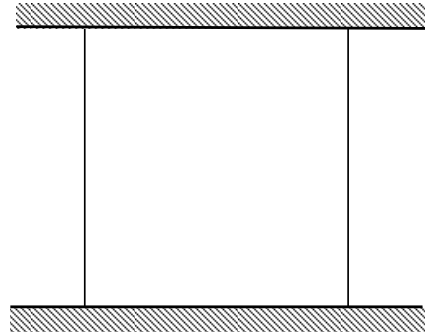
b = 20 INCHES

d = 204 INCHES



h = 138.5

CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED

l = 14149440 in4
 h = 138.5 in
 d = 204 in
 A = 4080 in2

P = 1 lbs
 Em = 5098235 psi
 Er = 2124265 psi

$\Delta_m = 3.06909E-09$

$\Delta_t = 2.22453E-08$

$\Delta_v = 1.91762E-08$

R = 44953372.4

Wall Rigidity

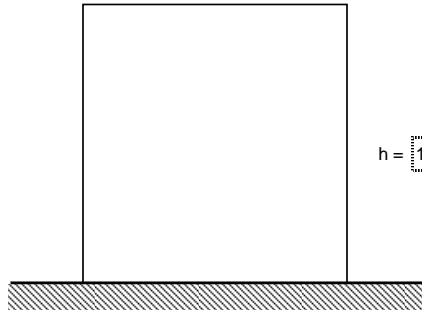
WALL (OR PORTION THEREOF) GEOMETRY

Transverse Direction



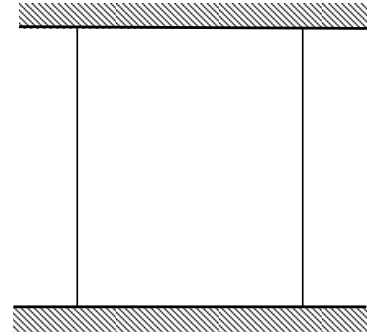
d = 204 INCHES

b = 20 INCHES



h = 138.5

CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: FIXED	
l = 136000 in ⁴	P = 1 lbs
h = 138.5 in	Em = 5098235 psi
d = 20 in	Er = 2124265 psi
A = 4080 in ²	

$\Delta_m =$	3.19308E-07	$\Delta_t =$	3.38E-07
$\Delta_v =$	1.91762E-08	R =	2954351

Wall Rigidity

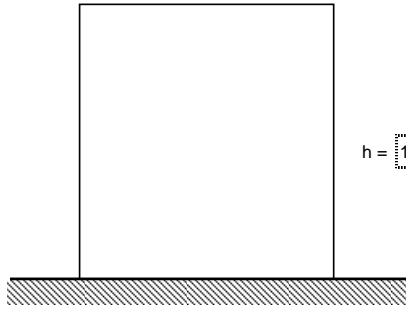
WALL (OR PORTION THEREOF) GEOMETRY

Longitudinal Direction

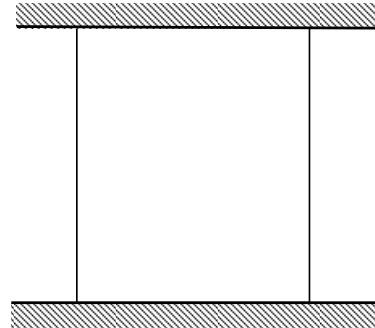


b = 20 INCHES

d = 162.5 INCHES



CANTILEVER



FIXED

h = 134

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \quad (\text{CANTILEVER})$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \quad (\text{FIXED})$$

Input

END FIXITY: CANTILEVER

l = 7151692.7 in⁴
 h = 134 in
 d = 162.5 in
 A = 3250 in²

P = 1 lbs
 E_m = 5098235 psi
 E_v = 2124265 psi

$\Delta_m = 2.19971\text{E-}08$

$\Delta_t = 4.5288\text{E-}08$

$\Delta_v = 2.32913\text{E-}08$

R = 22080724.6

Wall Rigidity

WALL (OR PORTION THEREOF) GEOMETRY

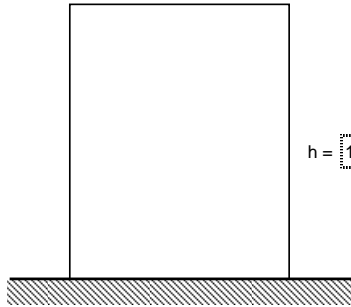
Transverse Direction



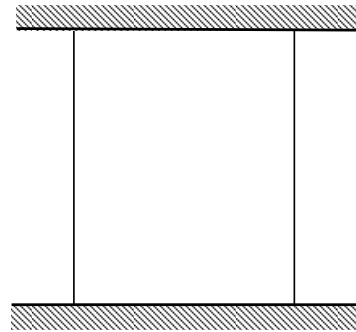
$$d = 162.5 \text{ INCHES}$$

$$b = 20 \text{ INCHES}$$

$$h = 134$$



CANTILEVER



FIXED

$$R = \frac{1}{\Delta}$$

$$\Delta_t = \Delta_v + \Delta_m$$

$$\Delta_m = \frac{Ph^3}{3E_m I} \text{ (CANTILEVER)}$$

$$\Delta_v = \frac{1.2Ph}{E_v A}$$

$$= \frac{Ph^3}{12E_m I} \text{ (FIXED)}$$

Input

END FIXITY: CANTILEVER	
$I = 108333.3 \text{ in}^4$	$P = 1 \text{ lbs}$
$h = 134 \text{ in}$	$E_m = 5098235 \text{ psi}$
$d = 20 \text{ in}$	$E_r = 2124265 \text{ psi}$
$A = 3250 \text{ in}^2$	

$\Delta_m = 1.45E-06$	$\Delta_t = 1.48E-06$
$\Delta_v = 2.33E-08$	$R = 677763.8$

APPENDIX 4

SHEAR WALL 304 F		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_C =$ 5098235		$E_R =$ 2124265
5		
	46917139	1038543
4		
	57476765	3606046
3		
	57476765	3606046
2		
	12193341	4657588
1		
	5680042	887027
D		
	18736682	3944636
E		
	19709897	4228058
F		
TOTAL	2390714	298543

APPENDIX 4

SHEAR WALL 304 G		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_C =$ 5098235		$E_R =$ 2124265
5	56720213	1176181
4	66515939	4083956
3	66515939	4083956
2	66515939	4083956
1	56337941	2711710
D	18126098	4641479
E	19709897	4228058
F		
TOTAL	5365169	415709

APPENDIX 4

SHEAR WALL 304 H		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_c =$ 5098235		$E_r =$ 2124265
5	2.2E+07	677764
4	4.5E+07	2954351
3	4.5E+07	2954351
2	4.5E+07	2954351
1	4.5E+07	2954351
D	8212510	3385609
E	1.4E+07	3560470
F		
TOTAL	3039624	293634

APPENDIX 4

SHEAR WALL 304 J		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_C =$ 5098235		$E_R =$ 2124265
5		
	65203092	1940807
4		
	76294906	6417518
3		
	76294906	6417518
2		
	76294906	6417518
1		
	5917378	3411274
D		
	26780001	8261640
E		
	23651876	6985704
F		
TOTAL	3297361	649327

APPENDIX 4

SHEAR WALL 304 K		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LB)	(TR)
$E_C =$ 5098235		$E_R =$ 2124265
5		
	68064255	2018421
4		
	79819126	6885439
3		
	79819126	6885439
2		
	79819126	6885439
1		
	6732535	3477316
D		
	22654849	6606520
E		
	23651876	6985704
F		
TOTAL	3481530	660837

SHEAR WALL 304 R

RIGIDITY SUMMARY

$f'_c = 8000$

RIGIDITY

LVL	(LG)	(TR)
	Ec = 5098235	Er = 2124265
5		
	56720213	1176181
4		
	66515939	4083956
3		
	66515939	4083956
2		
	66515939	4083956
1		
	10001714	2916410
D		
	15935609	3338467
E		
	19709897	4228058
F		
TOTAL	3620516	405888

APPENDIX 4

SHEAR WALL 304 S		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_c =$ 5098235		$E_r =$ 2124265
5	56720213	1176181
4	66515939	4083956
3	66515939	4083956
2	66515939	4083956
1	10544905	3126627
D	13385344	2584207
E	19709897	4228058
F		
TOTAL	3533451.4	395553

APPENDIX 4

SHEAR WALL 304 T		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LB)	(TR)
	$E_C =$ 5098235	$E_R =$ 2124265
5		
	34282126	1504935
4		
	54883740	5442032
3		
	54883740	5442032
2		
	54883740	5442032
1		
	54883740	5442032
D		
	54883740	5442032
E		
	85591134	15074414
F		
TOTAL	7578351	606210.4

APPENDIX 4

SHEAR WALL 304 U		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_c =$ 5098235		$E_r =$ 2124265
5	47793304	6033171
4	60994637	5493701
3	60994637	5493701
2	60994637	5493701
1	60994637	5493701
D	60994637	5493701
E	89963482	13787574
F		
TOTAL	8770893	870767.2

APPENDIX 4

SHEAR WALL 305 F		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LB)	(TR)
$E_c =$ 5098235		$E_r =$ 2124265
5		
	47793304	6033171
4		
	60994637	5493701
3		
	60994637	5493701
2		
	60994637	5493701
1		
	60994637	5493701
D		
	60994637	5493701
E		
	89963482	13787574
F		
TOTAL	8770893	870767.2

APPENDIX 4

SHEAR WALL 305 @		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_c =$ 5098235		$E_r =$ 2124265
5		
	68064255	7561574
4		
	79819126	6885439
3		
	79819126	6885439
2		
	79819126	6885439
1		
	79819126	6885439
D		
	79819126	6885439
E		
	1.15E+08	17280426
F		
TOTAL	11628239	1091362

APPENDIX 4

SHEAR WALL 305 H		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_C =$ 5098235		$E_R =$ 2124265
5	68064255	7561574
4	79819126	6885439
3	79819126	6885439
2	79819126	6885439
1	79819126	6885439
D	12298667	4296565
E	23651876	6985704
F		
TOTAL	5308000	918208.5

APPENDIX 4

SHEAR WALL 305 J		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_c = 5098234.99$		$E_r = 2124265$
5	68064255.4	7561574
4	79819126.5	6885439
3	79819126.5	6885439
2	79819126.5	6885439
1	79819126.5	6885439
D	79819126.5	6885439
E	115421256	17280426
F		
TOTAL	11628239.3	1091362

APPENDIX 4

SHEAR WALL 305 K		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_c =$ 5098235		$E_r =$ 2124265
5		
	68064255	7561574
4		
	79819126	6885439
3		
	79819126	6885439
2		
	79819126	6885439
1		
	15076584	6885439
D		
	12298667	4296565
E		
	23651876	6985704
F		
TOTAL	4128912	918208.5

APPENDIX 4

SHEAR WALL 305 R		
<i>RIGIDITY SUMMARY</i>		$f'_c =$ 8000
RIGIDITY		
LVL	(LG)	(TR)
$E_c =$ 5098235		$E_r =$ 2124265
5	33259187	1478028
4	47238886	5041997
3	47238886	5041997
2	47238886	5041997
1	47238886	5041997
D	47238886	5041997
E	72105622	1.3E+07
F		
TOTAL	6676440	572320

APPENDIX 7

Level E					
SHEAR WALL	RIGIDITY		ORIENTATION	RIGIDITY	
	(LG)	(TR)		X-DIRECTION	Y-DIRECTION
304 F	19709896.9	4228057.71	270	4228057.71	19709896.88
304 G	19709896.9	4228057.71	0	19709896.88	4228057.71
304 H	13707430.4	3560469.65	270	3560469.65	13707430.38
304 J	23651876.3	6985704.28	0	23651876.25	6985704.28
304 K	23651876.3	6985704.28	0	23651876.25	6985704.28
304 R	19709896.9	4228057.71	270	4228057.71	19709896.88
304 S	19709896.9	4228057.71	270	4228057.71	19709896.88
304 T	85591133.9	15074414.5	270	15074414.49	85591133.91
304 U	89963481.6	13787574.2	0	89963481.65	13787574.23
305 F	89963481.6	13787574.2	0	89963481.65	13787574.23
305 G	115421256	17280426.4	45	93834259.70	93834259.70
305 H	23651876.3	6985704.28	135	21664040.95	21664040.95
305 J	115421256	17280426.4	45	93834259.70	93834259.70
305 K	23651876.3	6985704.28	135	21664040.95	21664040.95
305 R	72105621.8	12653929.2	45	59934053.34	59934053.34
TOTAL				569190324.5883	495133524.2833

Level E		
SHEAR WALL	RELATIVE RIGIDITY	
	X-DIRECTION	Y-DIRECTION
304 F	0.0074282	0.0398072
304 G	0.0346280	0.0085392
304 H	0.0062553	0.0276843
304 J	0.0415535	0.0141087
304 K	0.0415535	0.0141087
304 R	0.0074282	0.0398072
304 S	0.0074282	0.0398072
304 T	0.0264840	0.1728648
304 U	0.1580552	0.0278462
305 F	0.1580552	0.0278462
305 G	0.1648557	0.1895130
305 H	0.0380612	0.0437539
305 J	0.1648557	0.1895130
305 K	0.0380612	0.0437539
305 R	0.1052970	0.1210462
TOTAL		1.0000

SHEAR WALL	COORDINATES			WEIGHTED COORDINATES	
	X	Y		X	Y
304 F	3138	3151.75		1.3268E+10	6.2121E+10
304 G	3150.25	3096.25		6.2091E+10	1.3091E+10
304 H	768	2544.25		2734440693	3.4875E+10
304 J	1638	2736.25		3.8742E+10	1.9115E+10
304 K	2148	2736.25		5.0804E+10	1.9115E+10
304 R	1752	2364.25		7407557112	4.6599E+10
304 S	2136	2364.25		9031131274	4.6599E+10
304 T	3138	1644.25		4.7304E+10	1.4073E+11
304 U	3026	2016.25		2.7223E+11	2.7799E+10
305 F	3026	1656.25		2.7223E+11	2.2836E+10
305 G	2015.125	748.6562		1.8909E+11	7.025E+10
305 H	2366.2187	380.5937		5.1262E+10	8245197502
305 J	2524.25	239.5312		2.3686E+11	2.2476E+10
305 K	2183.7812	198.1562		4.731E+10	4292864031
305 R	2911.375	-319.4375		1.7449E+11	-1.9145E+10
TOTAL				1.4749E+12	5.19E+11

BUILDING TORSIONAL MOMENT OF INERTIA & SHEAR DISTRIBUTION					
D _{x1}	D _{y1}	R _y * D _{x1} ²	R _x * D _{y1} ²	(R _y * D _{x1} ²)/J	(R _x * D _{y1} ²)/J
546.86	2103.55	5.89438E+12	1.87087E+13	1.45545E-05	1.201E-05
559.11	2048.05	1.32171E+12	8.2673E+13	3.19209E-06	5.4508E-05
-1823.14	1496.05	4.55612E+13	7.96887E+12	-3.37452E-05	7.1926E-06
-953.14	1688.05	6.34633E+12	6.7396E+13	-8.99089E-06	5.3912E-05
-443.14	1688.05	1.3718E+12	6.7396E+13	-4.1801E-06	5.3912E-05
-839.14	1316.05	1.38788E+13	7.32289E+12	-2.23334E-05	7.5136E-06
-455.14	1316.05	4.08293E+12	7.32289E+12	-1.21134E-05	7.5136E-06
546.86	596.05	2.55966E+13	5.35549E+12	6.32037E-05	1.2133E-05
434.86	968.05	2.60729E+12	8.43059E+13	8.09608E-06	0.0001176
434.86	608.05	2.60729E+12	3.32612E+13	8.09608E-06	7.3865E-05
-576.01	-299.55	3.11335E+13	8.41968E+12	-7.29845E-05	-3.7955E-05
-224.92	-667.61	1.09596E+12	9.65576E+12	-6.57967E-06	-1.953E-05
-66.89	-808.67	4.19826E+11	6.13632E+13	-8.47523E-06	-0.00010246
-407.36	-850.05	3.59494E+12	1.56541E+13	-1.19166E-05	-2.4867E-05
320.24	-1367.64	6.14631E+12	1.12103E+14	2.59167E-05	-0.00011068
		1.51659E+14	5.88907E+14	-5.82598E-05	0.00010466
J =		7.4E+14			

LEVEL E SUMMARY					
	RIGIDITY			CENTER OF RIGIDITY	
	X	Y		X	Y
	569190324.5883	495133524.2833		2591.13885	1048.20459

APPENDIX 7

LEVEL D					
SHEAR WALL	RIGIDITY		ORIENTATION	RIGIDITY	
	(LG)	(TR)		X-DIRECTION	Y-DIRECTION
304 F	18736681.8	3944636.44	270	3944636.44	18736681.76
304 G	18126098.4	4641479.47	0	18126098.39	4641479.47
304 H	8212510.01	3385609.38	270	3385609.38	8212510.01
304 J	26780000.7	8261640.45	0	26780000.72	8261640.45
304 K	22654848.7	6606519.95	0	22654848.71	6606519.95
304 R	15935608.8	3338467.15	270	3338467.15	15935608.80
304 S	13385343.6	2584206.78	270	2584206.78	13385343.60
304 T	54883740.2	5442032.32	270	5442032.32	54883740.20
304 U	60994636.8	5493701.48	0	60994636.81	5493701.48
305 F	60994636.8	5493701.48	0	60994636.81	5493701.48
305 G	79819126.5	6885439.18	45	61309386.35	61309386.35
305 H	12298666.5	4296564.82	135	11734600.62	11734600.62
305 J	79819126.5	6885439.18	45	61309386.35	61309386.35
305 K	12298666.5	4296564.82	135	11734600.62	11734600.62
305 R	47238886.2	5041997.13	45	36968167.10	36968167.10
TOTAL				391301314.5586	324707068.2132

LEVEL D		
SHEAR WALL	RELATIVE RIGIDITY	
	X-DIRECTION	Y-DIRECTION
304 F	0.0100808	0.0577033
304 G	0.0463226	0.0142944
304 H	0.0086522	0.0252921
304 J	0.0684383	0.0254434
304 K	0.0578962	0.0203461
304 R	0.0085317	0.0490769
304 S	0.0066041	0.0412228
304 T	0.0139075	0.1690254
304 U	0.1558764	0.0169189
305 F	0.1558764	0.0169189
305 G	0.1566808	0.1888144
305 H	0.0299887	0.0361390
305 J	0.1566808	0.1888144
305 K	0.0299887	0.0361390
305 R	0.0944749	0.1138508
TOTAL		1.0000

SHEAR WALL	COORDINATES		WEIGHTED COORDINATES
	X	Y	
304 F	3138	3151.75	1.2378E+10 5.9053E+10
304 G	3150.25	3096.25	5.7102E+10 1.4371E+10
304 H	768	2544.25	2600148007 2.0895E+10
304 J	1638	2736.25	4.3866E+10 2.2606E+10
304 K	2148	2736.25	4.8663E+10 1.8077E+10
304 R	1752	2364.25	5848994453 3.7676E+10
304 S	2136	2364.25	5519865687 3.1646E+10
304 T	3138	1644.25	1.7077E+10 9.0243E+10
304 U	3026	2016.25	1.8457E+11 1.1077E+10
305 F	3026	1656.25	1.8457E+11 9098943069
305 G	2015.125	748.6562	1.2355E+11 4.59E+10
305 H	2366.2187	380.5937	2.7767E+10 4466115067
305 J	2524.25	239.5312	1.5476E+11 1.4686E+10
305 K	2183.7812	198.1562	2.5626E+10 2325283867
305 R	2911.375	-319.4375	1.0763E+11 -1.1809E+10
TOTAL			1.0015E+12 3.7031E+11

BUILDING TORSIONAL MOMENT OF INERTIA & SHEAR DISTRIBUTION					
D _{x1}	D _{y1}	R _y * D _{x1} ²	R _x * D _{y1} ²	(R _y * D _{x1} ²)/J	(R _x * D _{y1} ²)/J
578.54	2011.31	6.2713E+12	1.5957E+13	1.98125E-05	1.4501E-05
590.79	1955.81	1.62E+12	6.9336E+13	5.0119E-06	6.4796E-05
-1791.46	1403.81	2.6357E+13	6.6719E+12	-2.689E-05	8.6868E-06
-921.46	1595.81	7.0149E+12	6.8198E+13	-1.3914E-05	7.811E-05
-411.46	1595.81	1.1185E+12	5.7693E+13	-4.9684E-06	6.6078E-05
-807.46	1223.81	1.039E+13	5E+12	-2.3518E-05	7.4675E-06
-423.46	1223.81	2.4003E+12	3.8704E+12	-1.036E-05	5.7804E-06
578.54	503.81	1.837E+13	1.3813E+12	5.8035E-05	5.0112E-06
466.54	875.81	1.1957E+12	4.6785E+13	4.68454E-06	9.7637E-05
466.54	515.81	1.1957E+12	1.6228E+13	4.68454E-06	5.7503E-05
-544.34	-391.79	1.8166E+13	9.4108E+12	-6.0997E-05	-4.39E-05
-193.24	-759.85	4.382E+11	6.7752E+12	-4.1446E-06	-1.63E-05
-35.21	-900.91	7.6017E+10	4.9761E+13	-3.9458E-06	-0.000101
-375.68	-942.29	1.6562E+12	1.0419E+13	-8.0575E-06	-2.021E-05
351.91	-1459.88	4.5782E+12	7.8788E+13	2.37781E-05	-9.864E-05
		1.0085E+14	4.4628E+14	-4.079E-05	0.00012556
J =		5.5E+14			

LEVEL D SUMMARY					
	RIGIDITY			CENTER OF RIGIDITY	
	X	Y		X	Y
	391301314.5586	324707068.2132		2559.46198	1140.44334

APPENDIX 7

Level 1					
SHEAR WALL	RIGIDITY		ORIENTATION	RIGIDITY	
	(LG)	(TR)		X-DIRECTION	Y-DIRECTION
304 F	5680041.9	887026.96	270	887026.96	5680041.90
304 G	56337940.6	27111709.56	0	56337940.63	27111709.56
304 H	44953372.4	2954350.86	270	2954350.86	44953372.40
304 J	5917378.13	3411274.13	0	5917378.13	3411274.13
304 K	6732534.97	3477315.7	0	6732534.97	3477315.70
304 R	10001714.3	2916409.9	270	2916409.90	10001714.30
304 S	10544905	3126627.48	270	3126627.48	10544904.98
304 T	54883740.2	5442032.32	270	5442032.32	54883740.20
304 U	60994636.8	5493701.48	0	60994636.81	5493701.48
305 F	60994636.8	5493701.48	0	60994636.81	5493701.48
305 G	79819126.5	6885439.18	45	61309386.35	61309386.35
305 H	79819126.5	6885439.18	135	61309386.35	61309386.35
305 J	79819126.5	6885439.18	45	61309386.35	61309386.35
305 K	15076584.2	6885439.18	135	15529495.68	15529495.68
305 R	47238886.2	5041997.13	45	36968167.10	36968167.10
TOTAL				442729396.7165	383077297.9721

Level 1		
SHEAR WALL	RELATIVE RIGIDITY	
	X-DIRECTION	Y-DIRECTION
304 F	0.0020035	0.0148274
304 G	0.1272514	0.0070788
304 H	0.0066730	0.1173480
304 J	0.0133657	0.0089049
304 K	0.0152069	0.0090773
304 R	0.0065873	0.0261089
304 S	0.0070622	0.0275268
304 T	0.0122920	0.1432707
304 U	0.1377696	0.0143410
305 F	0.1377696	0.0143410
305 G	0.1384805	0.1600444
305 H	0.1384805	0.1600444
305 J	0.1384805	0.1600444
305 K	0.0350767	0.0405388
305 R	0.0835006	0.0965032
TOTAL		1.0000

SHEAR WALL	COORDINATES			WEIGHTED COORDINATES	
	X	Y		X	Y
304 F	3138	3151.75		2783490600	1.7902E+10
304 G	3150.25	3096.25		1.7748E+11	8396130722
304 H	768	2544.25		2268941463	1.1437E+11
304 J	1638	2736.25		9692665369	9334098843
304 K	2148	2736.25		1.4461E+10	9514805094
304 R	1752	2364.25		5109550151	2.3647E+10
304 S	2136	2364.25		6678476302	2.4931E+10
304 T	3138	1644.25		1.7077E+10	9.0243E+10
304 U	3026	2016.25		1.8457E+11	1.1077E+10
305 F	3026	1656.25		1.8457E+11	9098943069
305 G	2015.125	748.6562		1.2355E+11	4.59E+10
305 H	2366.2187	380.5937		1.4507E+11	2.3334E+10
305 J	2524.25	239.5312		1.5476E+11	1.4686E+10
305 K	2183.7812	198.1562		3.3913E+10	3077265853
305 R	2911.375	-319.4375		1.0763E+11	-1.1809E+10
TOTAL				1.1696E+12	3.937E+11

BUILDING TORSIONAL MOMENT OF INERTIA & SHEAR DISTRIBUTION					
D _{x1}	D _{y1}	R _x * D _{x1} ²	R _x * D _{y1} ²	(R _y * D _{x1} ²)/J	(R _x * D _{y1} ²)/J
496.19	2124.01	1.3984E+12	4.0018E+12	3.7223E-06	2.4883E-06
508.44	2068.51	7.01E+11	2.4106E+14	1.8209E-06	0.00015391
-1873.81	1516.51	1.5784E+14	6.7945E+12	-0.00011125	5.9173E-06
-1003.81	1708.51	3.4373E+12	1.7273E+13	-4.5225E-06	1.3352E-05
-493.81	1708.51	8.4795E+11	1.9652E+13	-2.2679E-06	1.5192E-05
-889.81	1336.51	7.919E+12	5.2095E+12	-1.1754E-05	5.148E-06
-505.81	1336.51	2.6979E+12	5.585E+12	-7.0444E-06	5.519E-06
496.19	616.51	1.3512E+13	2.0685E+12	3.5967E-05	4.4311E-06
384.19	988.51	8.1086E+11	5.9601E+13	2.7875E-06	7.9632E-05
384.19	628.51	8.1086E+11	2.4095E+13	2.7875E-06	5.0631E-05
-626.69	-279.08	2.4079E+13	4.7751E+12	-5.0745E-05	-2.2598E-05
-275.60	-647.14	4.6566E+12	2.5676E+13	-2.2316E-05	-5.2401E-05
-117.56	-788.21	8.4738E+11	3.809E+13	-9.5195E-06	-6.3823E-05
-458.03	-829.58	3.258E+12	1.0687E+13	-9.3944E-06	-1.7015E-05
269.56	-1347.17	2.6862E+12	6.7093E+13	1.3161E-05	-6.5776E-05
		2.255E+14	5.3166E+14	-0.00016857	0.00011461
J =		7.6E+14			

LEVEL 1 SUMMARY					
	RIGIDITY			CENTER OF RIGIDITY	
	X	Y		X	Y
	442729396.7165	383077297.9721		2641.81413	1027.73685

APPENDIX 7

Level 2					
SHEAR WALL	RIGIDITY		ORIENTATION	RIGIDITY	
	(LG)	(TR)		X-DIRECTION	Y-DIRECTION
304 F	12193341.1	4657587.7	270	4657587.70	12193341.08
304 G	66515938.7	4083955.6	0	66515938.75	4083955.60
304 H	44953372.4	2954350.86	270	2954350.86	44953372.40
304 J	76294906.2	6417517.75	0	76294906.24	6417517.75
304 K	79819126.5	6885439.18	0	79819126.50	6885439.18
304 R	66515938.7	4083955.6	270	4083955.60	66515938.75
304 S	66515938.7	4083955.6	270	4083955.60	66515938.75
304 T	54883740.2	5442032.32	270	5442032.32	54883740.20
304 U	60994636.8	5493701.48	0	60994636.81	5493701.48
305 F	60994636.8	5493701.48	0	60994636.81	5493701.48
305 G	79819126.5	6885439.18	45	61309386.35	61309386.35
305 H	79819126.5	6885439.18	135	61309386.35	61309386.35
305 J	79819126.5	6885439.18	45	61309386.35	61309386.35
305 K	79819126.5	6885439.18	135	61309386.35	61309386.35
305 R	47238886.2	5041997.13	45	36968167.10	36968167.10
TOTAL				648046839.7086	555642359.1820

Level 2		
SHEAR WALL	RELATIVE RIGIDITY	
	X-DIRECTION	Y-DIRECTION
304 F	0.0071871	0.0219446
304 G	0.1026406	0.0073500
304 H	0.0045589	0.0809034
304 J	0.1177305	0.0115497
304 K	0.1231688	0.0123919
304 R	0.0063019	0.1197100
304 S	0.0063019	0.1197100
304 T	0.0083976	0.0987753
304 U	0.0941207	0.0098871
305 F	0.0941207	0.0098871
305 G	0.0946064	0.1103397
305 H	0.0946064	0.1103397
305 J	0.0946064	0.1103397
305 K	0.0946064	0.1103397
305 R	0.0570455	0.0665323
TOTAL		1.0000

SHEAR WALL	COORDINATES			WEIGHTED COORDINATES	
	X	Y		X	Y
304 F	3138	3151.75		1.4616E+10	3.843E+10
304 G	3150.25	3096.25		2.0954E+11	1.2645E+10
304 H	768	2544.25		2268941463	1.1437E+11
304 J	1638	2736.25		1.2497E+11	1.756E+10
304 K	2148	2736.25		1.7145E+11	1.884E+10
304 R	1752	2364.25		7155090220	1.5726E+11
304 S	2136	2364.25		8723329172	1.5726E+11
304 T	3138	1644.25		1.7077E+10	9.0243E+10
304 U	3026	2016.25		1.8457E+11	1.1077E+10
305 F	3026	1656.25		1.8457E+11	9098943069
305 G	2015.125	748.6562		1.2355E+11	4.59E+10
305 H	2366.2187	380.5937		1.4507E+11	2.3334E+10
305 J	2524.25	239.5312		1.5476E+11	1.4686E+10
305 K	2183.7812	198.1562		1.3389E+11	1.2149E+10
305 R	2911.375	-319.4375		1.0763E+11	-1.1809E+10
TOTAL				1.5898E+12	7.1105E+11

BUILDING TORSIONAL MOMENT OF INERTIA & SHEAR DISTRIBUTION					
D _{x1}	D _{y1}	R _y * D _{x1} ²	R _x * D _{y1} ²	(R _y * D _{x1} ²)/J	(R _x * D _{y1} ²)/J
684.73	1872.07	5.7169E+12	1.6323E+13	7.21595E-06	7.53592E-06
696.98	1816.57	1.9839E+12	2.195E+14	2.4601E-06	0.000104431
-1685.27	1264.57	1.2767E+14	4.7244E+12	-6.54767E-05	3.22892E-06
-815.27	1456.57	4.2655E+12	1.6187E+14	-4.52193E-06	9.60462E-05
-305.27	1456.57	6.4167E+11	1.6934E+14	-1.81666E-06	0.000100483
-701.27	1084.57	3.2712E+13	4.8039E+12	-4.0315E-05	3.82817E-06
-317.27	1084.57	6.6957E+12	4.8039E+12	-1.82395E-05	3.82817E-06
684.73	364.57	2.5732E+13	7.233E+11	3.24799E-05	1.71472E-06
572.73	736.57	1.802E+12	3.3092E+13	2.71936E-06	3.88292E-05
572.73	376.57	1.802E+12	8.6492E+12	2.71936E-06	1.98513E-05
-438.15	-531.03	1.177E+13	1.7289E+13	-2.32168E-05	-2.81382E-05
-87.05	-899.09	4.6463E+11	4.956E+13	-4.61289E-06	-4.76413E-05
70.98	-1040.15	3.0886E+11	6.6332E+13	3.76094E-06	-5.5116E-05
-269.49	-1081.53	4.4527E+12	7.1714E+13	-1.428E-05	-5.73084E-05
458.10	-1599.12	7.758E+12	9.4534E+13	1.46367E-05	-5.10932E-05
		2.3378E+14	9.2325E+14	-0.000106487	0.00014048
J = 1.2E+15					

LEVEL 2 SUMMARY					
	RIGIDITY			CENTER OF RIGIDITY	
	X	Y		X	Y
	648046839.7086	555642359.1820		2453.27341	1279.6827

APPENDIX 7

Level 3					
SHEAR WALL	RIGIDITY		ORIENTATION	RIGIDITY	
	(LG)	(TR)		X-DIRECTION	Y-DIRECTION
304 F	57476764.9	3606045.91	270	3606045.91	57476764.95
304 G	66515938.7	4083955.6	0	66515938.75	4083955.60
304 H	44953372.4	2954350.86	270	2954350.86	44953372.40
304 J	76294906.2	6417517.75	0	76294906.24	6417517.75
304 K	79819126.5	6885439.18	0	79819126.50	6885439.18
304 R	66515938.7	4083955.6	270	4083955.60	66515938.75
304 S	66515938.7	4083955.6	270	4083955.60	66515938.75
304 T	54883740.2	5442032.32	270	5442032.32	54883740.20
304 U	60994636.8	5493701.48	0	60994636.81	5493701.48
305 F	60994636.8	5493701.48	0	60994636.81	5493701.48
305 G	79819126.5	6885439.18	45	61309386.35	61309386.35
305 H	79819126.5	6885439.18	135	61309386.35	61309386.35
305 J	79819126.5	6885439.18	45	61309386.35	61309386.35
305 K	79819126.5	6885439.18	135	61309386.35	61309386.35
305 R	47238886.2	5041997.13	45	36968167.10	36968167.10
TOTAL				646995297.9170	600925783.0438

Level 3		
SHEAR WALL	RELATIVE RIGIDITY	
	X-DIRECTION	Y-DIRECTION
304 F	0.0055735	0.0956470
304 G	0.1028075	0.0067961
304 H	0.0045663	0.0748069
304 J	0.1179219	0.0106794
304 K	0.1233689	0.0114581
304 R	0.0063122	0.1106891
304 S	0.0063122	0.1106891
304 T	0.0084112	0.0913320
304 U	0.0942737	0.0091421
305 F	0.0942737	0.0091421
305 G	0.0947602	0.1020249
305 H	0.0947602	0.1020249
305 J	0.0947602	0.1020249
305 K	0.0947602	0.1020249
305 R	0.0571382	0.0615187
TOTAL	1.0000	1.0000

SHEAR WALL	COORDINATES			WEIGHTED COORDINATES	
	X	Y		X	Y
304 F	3138	3151.75		1.1316E+10	1.8115E+11
304 G	3150.25	3096.25		2.0954E+11	1.2645E+10
304 H	768	2544.25		2268941463	1.1437E+11
304 J	1638	2736.25		1.2497E+11	1.756E+10
304 K	2148	2736.25		1.7145E+11	1.884E+10
304 R	1752	2364.25		7155090220	1.5726E+11
304 S	2136	2364.25		8723329172	1.5726E+11
304 T	3138	1644.25		1.7077E+10	9.0243E+10
304 U	3026	2016.25		1.8457E+11	1.1077E+10
305 F	3026	1656.25		1.8457E+11	9098943069
305 G	2015.125	748.6562		1.2355E+11	4.59E+10
305 H	2366.2187	380.5937		1.4507E+11	2.3334E+10
305 J	2524.25	239.5312		1.5476E+11	1.4686E+10
305 K	2183.7812	198.1562		1.3389E+11	1.2149E+10
305 R	2911.375	-319.4375		1.0763E+11	-1.1809E+10
TOTAL				1.5865E+12	8.5377E+11

BUILDING TORSIONAL MOMENT OF INERTIA & SHEAR DISTRIBUTION					
D _{x1}	D _{y1}	R _y * D _{x1} ²	R _x * D _{y1} ²	(R _y * D _{x1} ²)/J	(R _x * D _{y1} ²)/J
685.84	1731.00	2.7036E+13	1.0805E+13	3.45198E-05	5.46615E-06
698.09	1675.50	1.9902E+12	1.8673E+14	2.49658E-06	9.7594E-05
-1684.16	1123.50	1.2751E+14	3.7291E+12	-6.62979E-05	2.90661E-06
-814.16	1315.50	4.2539E+12	1.3203E+14	-4.57542E-06	8.78899E-05
-304.16	1315.50	6.37E+11	1.3813E+14	-1.83395E-06	9.19497E-05
-700.16	943.50	3.2608E+13	3.6355E+12	-4.07828E-06	3.37423E-06
-316.16	943.50	6.6488E+12	3.6355E+12	-1.84157E-05	3.37423E-06
685.84	223.50	2.5816E+13	2.7183E+11	3.29625E-05	1.06508E-06
573.84	595.50	1.809E+12	2.163E+13	2.76064E-06	3.18071E-05
573.84	235.50	1.809E+12	3.3827E+12	2.76064E-06	1.25785E-05
-437.04	-672.10	1.171E+13	2.7694E+13	-2.34638E-05	-3.60839E-05
-85.94	-1040.16	4.5283E+11	6.6333E+13	-4.61408E-06	-5.58446E-05
72.09	-1181.22	3.1862E+11	8.5544E+13	3.87037E-06	-6.3418E-05
-268.38	-1222.60	4.416E+12	9.1642E+13	-1.44089E-05	-6.56394E-05
459.21	-1740.19	7.7958E+12	1.1195E+14	1.48661E-05	-5.63351E-05
		2.5481E+14	8.8714E+14	-8.01558E-05	6.06844E-05
J =		1.1E+15			

LEVEL 3 SUMMARY					
	RIGIDITY			CENTER OF RIGIDITY	
	X	Y		X	Y
	646995297.9170	600925783.0438		2452.16055	1420.75439

APPENDIX 7

Level 4					
SHEAR WALL	RIGIDITY		ORIENTATION	RIGIDITY	
	(LG)	(TR)		X-DIRECTION	Y-DIRECTION
304 F	57476764.9	3606045.91	270	3606045.91	57476764.95
304 G	66515938.7	4083955.6	0	66515938.75	4083955.60
304 H	44953372.4	2954350.86	270	2954350.86	44953372.40
304 J	76294906.2	6417517.75	0	76294906.24	6417517.75
304 K	79819126.5	6885439.18	0	79819126.50	6885439.18
304 R	66515938.7	4083955.6	270	4083955.60	66515938.75
304 S	66515938.7	4083955.6	270	4083955.60	66515938.75
304 T	54883740.2	5442032.32	270	5442032.32	54883740.20
304 U	60994636.8	5493701.48	0	60994636.81	5493701.48
305 F	60994636.8	5493701.48	0	60994636.81	5493701.48
305 G	79819126.5	6885439.18	45	61309386.35	61309386.35
305 H	79819126.5	6885439.18	135	61309386.35	61309386.35
305 J	79819126.5	6885439.18	45	61309386.35	61309386.35
305 K	79819126.5	6885439.18	135	61309386.35	61309386.35
305 R	47238886.2	5041997.13	45	36968167.10	36968167.10
TOTAL				646995297.9170	600925783.0438

Level 4		
SHEAR WALL	RELATIVE RIGIDITY	
	X-DIRECTION	Y-DIRECTION
304 F	0.0055735	0.0956470
304 G	0.1028075	0.0067961
304 H	0.0045663	0.0748069
304 J	0.1179219	0.0106794
304 K	0.1233689	0.0114581
304 R	0.0063122	0.1106891
304 S	0.0063122	0.1106891
304 T	0.0084112	0.0913320
304 U	0.0942737	0.0091421
305 F	0.0942737	0.0091421
305 G	0.0947602	0.1020249
305 H	0.0947602	0.1020249
305 J	0.0947602	0.1020249
305 K	0.0947602	0.1020249
305 R	0.0571382	0.0615187
TOTAL		1.0000

SHEAR WALL	COORDINATES			WEIGHTED COORDINATES	
	X	Y		X	Y
304 F	3138	3151.75		1.1316E+10	1.8115E+11
304 G	3150.25	3096.25		2.0954E+11	1.2645E+10
304 H	768	2544.25		2268941463	1.1437E+11
304 J	1638	2736.25		1.2497E+11	1.756E+10
304 K	2148	2736.25		1.7145E+11	1.884E+10
304 R	1752	2364.25		7155090220	1.5726E+11
304 S	2136	2364.25		8723329172	1.5726E+11
304 T	3138	1644.25		1.7077E+10	9.0243E+10
304 U	3026	2016.25		1.8457E+11	1.1077E+10
305 F	3026	1656.25		1.8457E+11	9098943069
305 G	2015.125	748.6562		1.2355E+11	4.59E+10
305 H	2366.2187	380.5937		1.4507E+11	2.3334E+10
305 J	2524.25	239.5312		1.5476E+11	1.4686E+10
305 K	2183.7812	198.1562		1.3389E+11	1.2149E+10
305 R	2911.375	-319.4375		1.0763E+11	-1.1809E+10
TOTAL				1.5865E+12	8.5377E+11

BUILDING TORSIONAL MOMENT OF INERTIA & SHEAR DISTRIBUTION					
D _{x1}	D _{y1}	R _y * D _{x1} ²	R _x * D _{y1} ²	(R _y * D _{x1} ²)/J	(R _x * D _{y1} ²)/J
685.84	1731.00	2.7036E+13	1.0805E+13	3.45198E-05	5.46615E-06
698.09	1675.50	1.9902E+12	1.8673E+14	2.49658E-06	9.7594E-05
-1684.16	1123.50	1.2751E+14	3.7291E+12	-6.6298E-05	2.90661E-06
-814.16	1315.50	4.2539E+12	1.3203E+14	-4.5754E-06	8.78899E-05
-304.16	1315.50	6.37E+11	1.3813E+14	-1.834E-06	9.19497E-05
-700.16	943.50	3.2608E+13	3.6355E+12	-4.0783E-05	3.37423E-06
-316.16	943.50	6.6488E+12	3.6355E+12	-1.8416E-05	3.37423E-06
685.84	223.50	2.5816E+13	2.7183E+11	3.29625E-05	1.06508E-06
573.84	595.50	1.809E+12	2.163E+13	2.76064E-06	3.18071E-05
573.84	235.50	1.809E+12	3.3827E+12	2.76064E-06	1.25785E-05
-437.04	-672.10	1.171E+13	2.7694E+13	-2.3464E-05	-3.60839E-05
-85.94	-1040.16	4.5283E+11	6.6333E+13	-4.6141E-06	-5.58446E-05
72.09	-1181.22	3.1862E+11	8.5544E+13	3.87037E-06	-6.3418E-05
-268.38	-1222.60	4.416E+12	9.1642E+13	-1.4409E-05	-6.56394E-05
459.21	-1740.19	7.7958E+12	1.1195E+14	1.48661E-05	-5.63351E-05
		2.5481E+14	8.8714E+14	-8.0156E-05	6.06844E-05
J = 1.1E+15					

LEVEL 4 SUMMARY					
	RIGIDITY			CENTER OF RIGIDITY	
	X	Y		X	Y
	646995297.9170	600925783.0438		2452.16055	1420.75439

APPENDIX 7

Level 5						
SHEAR WALL	RIGIDITY		ORIENTATION	RIGIDITY		
	(LG)	(TR)		X-DIRECTION	Y-DIRECTION	
304 F	46917138.8	1038542.63	270	1038542.63	46917138.80	
304 G	56720212.8	1176180.81	0	56720212.82	1176180.81	
304 H	22080724.6	677763.767	270	677763.77	22080724.63	
304 J	65203091.9	1940806.71	0	65203091.88	1940806.71	
304 K	68064255.4	2018420.81	0	68064255.39	2018420.81	
304 R	56720212.8	1176180.81	270	1176180.81	56720212.82	
304 S	56720212.8	1176180.81	270	1176180.81	56720212.82	
304 T	34282126.5	1504935.37	270	1504935.37	34282126.50	
304 U	47793304.4	6033171.05	0	47793304.39	6033171.05	
305 F	47793304.4	6033171.05	0	47793304.39	6033171.05	
305 G	68064255.4	7561574.38	45	53475537.06	53475537.06	
305 H	68064255.4	7561574.38	135	53475537.06	53475537.06	
305 J	68064255.4	7561574.38	45	53475537.06	53475537.06	
305 K	68064255.4	7561574.38	135	53475537.06	53475537.06	
305 R	33259186.9	1478028	45	24562920.22	24562920.22	
TOTAL				529612840.7545	472387234.4910	

Level 5			
SHEAR WALL	RELATIVE RIGIDITY		
	X-DIRECTION	Y-DIRECTION	
304 F	0.0019609	0.0993192	
304 G	0.1070975	0.0024899	
304 H	0.0012797	0.0467428	
304 J	0.1231146	0.0041085	
304 K	0.1285170	0.0042728	
304 R	0.0022208	0.1200714	
304 S	0.0022208	0.1200714	
304 T	0.0028416	0.0725721	
304 U	0.0902420	0.0127717	
305 F	0.0902420	0.0127717	
305 G	0.1009710	0.1132028	
305 H	0.1009710	0.1132028	
305 J	0.1009710	0.1132028	
305 K	0.1009710	0.1132028	
305 R	0.0463790	0.0519974	
TOTAL		1.0000	1.0000

SHEAR WALL	COORDINATES			WEIGHTED COORDINATES	
	X	Y		X	Y
304 F	3138	3151.75		3258946787	1.4787E+11
304 G	3150.25	3096.25		1.7868E+11	3641749848
304 H	768	2544.25		520522573	5.6179E+10
304 J	1638	2736.25		1.068E+11	5310532350
304 K	2148	2736.25		1.462E+11	5522903941
304 R	1752	2364.25		2060668788	1.341E+11
304 S	2136	2364.25		2512322221	1.341E+11
304 T	3138	1644.25		4722487194	5.6368E+10
304 U	3026	2016.25		1.4462E+11	1.2164E+10
305 F	3026	1656.25		1.4462E+11	9992439553
305 G	2015.125	748.6562		1.0776E+11	4.0035E+10
305 H	2366.2187	380.5937		1.2653E+11	2.0352E+10
305 J	2524.25	239.5312		1.3499E+11	1.2809E+10
305 K	2183.7812	198.1562		1.1678E+11	1.0597E+10
305 R	2911.375	-319.4375		7.1512E+10	-7846317829
TOTAL				1.2916E+12	6.412E+11

BUILDING TORSIONAL MOMENT OF INERTIA & SHEAR DISTRIBUTION					
D _{x1}	D _{y1}	R _y * D _{x1} ²	R _x * D _{y1} ²	(R _y * D _{x1})/J	(R _x * D _{y1})/J
699.28	1794.39	2.2942E+13	3.3439E+12	3.66988E-05	2.08455E-06
711.53	1738.89	5.9547E+11	1.7151E+14	9.36132E-07	0.000110327
-1670.72	1186.89	6.1634E+13	9.5478E+11	-4.12657E-05	8.99831E-07
-800.72	1378.89	1.2444E+12	1.2397E+14	-1.73834E-06	0.00010057
-290.72	1378.89	1.706E+11	1.2941E+14	-6.56389E-07	0.000104983
-686.72	1006.89	2.6749E+13	1.1925E+12	-4.35703E-05	1.32473E-06
-302.72	1006.89	5.1979E+12	1.1925E+12	-1.92067E-05	1.32473E-06
699.28	286.89	1.6764E+13	1.2387E+11	2.68157E-05	4.82957E-07
587.28	658.89	2.0808E+12	2.0749E+13	3.96333E-06	3.52251E-05
587.28	298.89	2.0808E+12	4.2697E+12	3.96333E-06	1.59791E-05
-423.60	-608.70	9.5954E+12	1.9814E+13	-2.53384E-05	-3.64108E-05
-72.50	-976.76	2.8111E+11	5.1019E+13	-4.33696E-06	-5.84273E-05
85.53	-1117.83	3.9117E+11	6.682E+13	5.11603E-06	-6.68653E-05
-254.94	-1159.20	3.4756E+12	7.1858E+13	-1.52499E-05	-6.93402E-05
472.65	-1676.80	5.4874E+12	6.9062E+13	1.29865E-05	-4.60714E-05
		1.5869E+14	7.3529E+14	-6.08828E-05	9.60868E-05
J = 8.9E+14					

LEVEL 5 SUMMARY					
	RIGIDITY			CENTER OF RIGIDITY	
	X	Y		X	Y
	529612840.7545	472387234.4910		2438.72229	1357.35758

TECHNICAL REPORT 3

APPENDIX E

LEVEL E

SHEAR FORCE			DISTRIBUTION OF SHEAR FORCES	
V = 5938			X	Y
304 F	0.007428	0.039807	44.11	236.38
304 G	0.034628	0.008539	205.63	50.71
304 H	0.006255	0.027684	37.15	164.39
304 J	0.041554	0.014109	246.75	83.78
304 K	0.041554	0.014109	246.75	83.78
304 R	0.007428	0.039807	44.11	236.38
304 S	0.007428	0.039807	44.11	236.38
304 T	0.026484	0.172865	157.27	1026.50
304 U	0.158055	0.027846	938.56	165.35
305 F	0.158055	0.027846	938.56	165.35
305 G	0.164856	0.189513	978.94	1125.36
305 H	0.038061	0.043754	226.01	259.82
305 J	0.164856	0.189513	978.94	1125.36
305 K	0.038061	0.043754	226.01	259.82
305 R	0.105297	0.121046	625.27	718.79
TOTAL	1.00000	1.00000		

TORSIONAL SHEAR FORCE			DISTRIBUTION OF SHEAR FORCES							
			X		Y					
			$E_x + E_{XADI}$	$E_x \cdot E_{XADI}$	$E_y + E_{YADI}$	$E_y \cdot E_{YADI}$				
			E = 145.8	E = -145.8	E = 229.2	E = -229.2				
		$V_T =$	865783	-865783	1361025.793	-1361025.793				
304 F	0.000012	0.000015	10.40	12.60	-10.40	16.35	19.81	-16.35	-19.81	
304 G	0.000055	0.000003	47.19	2.76	-47.19	-2.76	74.19	4.34	-74.19	-4.34
304 H	0.000007	-0.000034	6.23	-29.22	-6.23	29.22	9.79	-45.93	-9.79	45.93
304 J	0.000054	-0.000009	46.68	-7.78	-46.68	7.78	73.38	-12.24	-73.38	12.24
304 K	0.000054	-0.000004	46.68	-3.62	-46.68	3.62	73.38	-5.69	-73.38	5.69
304 R	0.000008	-0.000022	6.51	-19.34	-6.51	19.34	10.23	-30.40	-10.23	30.40
304 S	0.000008	-0.000012	6.51	-10.49	-6.51	10.49	10.23	-16.49	-10.23	16.49
304 T	0.000012	0.000063	10.50	54.72	-10.50	-54.72	16.51	86.02	-16.51	-86.02
304 U	0.000118	0.000008	101.81	7.01	-101.81	-7.01	160.05	11.02	-160.05	-11.02
305 F	0.000074	0.000008	63.95	7.01	-63.95	-7.01	100.53	11.02	-100.53	-11.02
305 G	-0.000038	-0.000073	-32.86	-63.19	32.86	63.19	-51.66	-99.33	51.66	99.33
305 H	-0.000020	-0.000007	-16.91	-5.70	16.91	5.70	-26.58	-8.96	26.58	8.96
305 J	-0.000102	-0.000008	-88.71	-7.34	88.71	7.34	-139.46	-11.54	139.46	11.54
305 K	-0.000025	-0.000012	-21.53	-10.32	21.53	10.32	-33.84	-16.22	33.84	16.22
305 R	-0.000111	0.000026	-95.83	22.44	95.83	-22.44	-150.64	35.27	150.64	-35.27

			DISTRIBUTION OF SHEAR FORCES							
			X		Y					
			$E_x + E_{XADI}$	$E_x \cdot E_{XADI}$	$E_y + E_{YADI}$	$E_y \cdot E_{YADI}$				
			E = -62	E = -353	E = 349	E = -109				
304 F			54.51	248.98	33.71	223.78	60.46	256.19	27.76	216.57
304 G			252.82	53.47	158.43	47.94	279.81	55.05	131.44	46.36
304 H			43.37	135.18	30.92	193.61	46.93	118.47	27.36	210.32
304 J			293.43	76.00	200.08	91.56	320.13	71.54	173.38	96.02
304 K			293.43	80.16	200.08	87.40	320.13	78.09	173.38	89.47
304 R			50.61	217.05	37.60	255.72	54.34	205.99	33.88	266.78
304 S			50.61	225.89	37.60	246.87	54.34	219.90	33.88	252.87
304 T			167.77	1081.22	146.76	971.78	173.78	1112.52	140.75	940.48
304 U			1040.37	172.36	836.74	158.35	1098.61	176.37	778.50	154.34
305 F			1002.51	172.36	874.61	158.35	1039.09	176.37	838.02	154.34
305 G			946.08	1062.17	1011.80	1188.55	927.28	1026.02	1030.60	1224.69
305 H			209.10	254.12	242.92	265.51	199.43	250.86	252.59	268.77
305 J			890.23	1118.02	1067.65	1132.70	839.48	1113.82	1118.40	1136.89
305 K			204.48	249.50	247.54	270.13	192.17	243.60	259.86	276.04
305 R			529.44	741.23	721.10	696.35	474.63	754.07	775.91	683.52

TECHNICAL REPORT 3

APPENDIX E

LEVEL D

SHEAR FORCE			DISTRIBUTION OF SHEAR FORCES	
V = 5938			X	Y
304 F	0.001961	0.099319	11.64	589.77
304 G	0.107098	0.002490	635.96	14.79
304 H	0.001280	0.046743	7.60	277.57
304 J	0.123115	0.004109	731.07	24.40
304 K	0.128517	0.004273	763.15	25.37
304 R	0.002221	0.120071	13.19	713.00
304 S	0.002221	0.120071	13.19	713.00
304 T	0.002842	0.072572	16.87	430.94
304 U	0.090242	0.012772	535.87	75.84
305 F	0.090242	0.012772	535.87	75.84
305 G	0.100971	0.113203	599.58	672.22
305 H	0.100971	0.113203	599.58	672.22
305 J	0.100971	0.113203	599.58	672.22
305 K	0.100971	0.113203	599.58	672.22
305 R	0.046379	0.051997	275.41	308.77
TOTAL	1.00000	1.00000		

TORSIONAL SHEAR FORCE			DISTRIBUTION OF SHEAR FORCES							
			X				Y			
			$E_x + E_{xADI}$		$E_x \cdot E_{xADI}$		$E_y + E_{yADI}$		$E_y \cdot E_{yADI}$	
			E = 692.034		E = 400.434		E = -595.516		E = -1053.92	
			4109407		2377840		-3536266.185		-6258317.771	
304 F	0.000015	0.000020	59.59	81.42	34.48	47.11	-51.28	-70.06	-90.75	-123.99
304 G	0.000065	0.000005	266.27	20.60	154.07	11.92	-229.13	-17.72	-405.51	-31.37
304 H	0.000009	-0.000027	35.70	-110.50	20.66	-63.94	-30.72	95.09	-54.36	168.29
304 J	0.000078	-0.000014	320.98	-57.18	185.73	-33.09	-276.22	49.20	-488.84	87.08
304 K	0.000066	-0.000005	271.54	-20.42	157.12	-11.81	-233.67	17.57	-413.54	31.09
304 R	0.000007	-0.000024	30.69	-96.65	17.76	-55.92	-26.41	83.17	-46.73	147.18
304 S	0.000006	-0.000010	23.75	-42.57	13.74	-24.63	-20.44	36.64	-36.18	64.84
304 T	0.000005	0.000058	20.59	238.49	11.92	138.00	-17.72	-205.23	-31.36	-363.20
304 U	0.000098	0.000005	401.23	19.25	232.17	11.14	-345.27	-16.57	-611.04	-29.32
305 F	0.000058	0.000005	236.30	19.25	136.73	11.14	-203.35	-16.57	-359.87	-29.32
305 G	#####	-0.000061	-180.41	-250.66	-104.39	-145.04	155.25	215.70	274.76	381.74
305 H	#####	-0.000004	-66.97	-17.03	-38.75	-9.86	57.63	14.66	101.99	25.94
305 J	#####	-0.000004	-414.86	-16.21	-240.05	-9.38	357.00	13.95	631.80	24.69
305 K	#####	-0.000008	-83.05	-33.11	-48.06	-19.16	71.47	28.49	126.48	50.43
305 R	#####	0.000024	-405.36	97.71	-234.55	56.54	348.82	-84.09	617.33	-148.81

DISTRIBUTION OF SHEAR FORCES										
			X				Y			
			$E_x + E_{xADI}$		$E_x \cdot E_{xADI}$		$E_y + E_{yADI}$		$E_y \cdot E_{yADI}$	
			E = -62		E = -353		E = 349		E = -109	
304 F			71.24	671.19	46.13	636.88	-39.64	519.71	-79.11	465.78
304 G			902.23	35.38	790.04	26.70	406.83	-2.94	230.45	-16.58
304 H			43.30	167.06	28.26	213.63	-23.12	372.66	-46.77	445.86
304 J			1052.06	-32.78	916.81	-8.69	454.86	73.60	242.24	111.48
304 K			1034.70	4.96	920.28	13.56	529.49	42.94	349.62	56.47
304 R			43.87	616.36	30.94	657.08	-13.22	796.17	-33.55	860.19
304 S			36.94	670.43	26.93	688.37	-7.25	749.64	-22.99	777.84
304 T			37.47	669.43	28.79	568.94	-0.85	225.72	-14.49	67.74
304 U			937.10	95.09	768.04	86.98	190.60	59.27	-75.17	46.52
305 F			772.18	95.09	672.60	86.98	332.52	59.27	176.00	46.52
305 G			419.17	421.55	495.19	527.17	754.83	887.92	874.34	1053.96
305 H			532.61	655.18	560.83	662.36	657.21	686.87	701.57	698.15
305 J			184.72	656.00	359.53	662.83	956.58	686.17	1231.38	696.91
305 K			516.53	639.10	551.53	653.06	671.05	700.71	726.06	722.64
305 R			-129.95	406.48	40.85	365.31	624.23	224.68	892.74	159.96

APPENDIX E

LEVEL		2	
SHEAR FORCE			
V =		5061	
		DISTRIBUTION OF SHEAR FORCES	
	X	Y	
304 F	0.007187	0.021945	36.37 111.06
304 B	0.102641	0.007350	519.45 37.20
304 H	0.004559	0.080903	23.07 409.44
304 J	0.117731	0.011550	595.82 58.45
304 K	0.123169	0.012392	623.34 62.71
304 R	0.006302	0.119710	31.89 605.83
304 S	0.006302	0.119710	31.89 605.83
304 T	0.008398	0.098775	42.50 499.89
304 U	0.094121	0.009887	476.33 50.04
305 F	0.094121	0.009887	476.33 50.04
305 B	0.094606	0.110340	478.79 558.41
305 H	0.094606	0.110340	478.79 558.41
305 J	0.094606	0.110340	478.79 558.41
305 K	0.094606	0.110340	478.79 558.41
305 R	0.057046	0.066532	288.70 336.71
TOTAL	1.00000	1.00000	

TORSIONAL SHEAR FORCE				DISTRIBUTION OF SHEAR FORCES							
				X				Y			
				$E_x + E_{yAD}$		$E_x - E_{yAD}$		$E_y + E_{xAD}$		$E_y - E_{xAD}$	
				E = 375.469973		E = 83.869973		E = -100.44539		E = -558.84539	
				1900198		424454		-508339.3224		-2828234.149	
				$V_T =$							
	X	Y									
304 F	0.000008	0.000007		14.32	13.71	3.20	3.06	-3.83	-3.67	-21.31	-20.41
304 B	0.000104	0.000002		198.44	4.67	44.33	1.04	-53.09	-1.25	-295.36	-6.96
304 H	0.000003	-0.000065		6.14	-124.42	1.37	-27.79	-1.64	33.28	-9.13	185.18
304 J	0.000096	-0.000005		182.51	-8.59	40.77	-1.92	-48.82	2.30	-271.64	12.79
304 K	0.000100	-0.000002		190.94	-3.45	42.65	-0.77	-51.08	0.92	-284.19	5.14
304 R	0.000004	-0.000040		7.27	-76.61	1.62	-17.11	-1.95	20.49	-10.83	114.02
304 S	0.000004	-0.000018		7.27	-34.66	1.62	-7.74	-1.95	9.27	-10.83	51.59
304 T	0.000002	0.000032		3.26	61.72	0.73	13.79	-0.87	-16.51	-4.85	-91.86
304 U	0.000039	0.000003		73.78	5.17	16.48	1.15	-19.74	-1.38	-109.82	-7.69
305 F	0.000020	0.000003		37.72	5.17	8.43	1.15	-10.09	-1.38	-56.14	-7.69
305 B	-0.000028	-0.000023		-53.47	-44.12	-11.94	-9.85	14.30	11.80	79.58	65.66
305 H	-0.000048	-0.000005		-90.53	-8.77	-20.22	-1.96	24.22	2.34	134.74	13.05
305 J	-0.000055	0.000004		-104.73	7.15	-23.39	1.60	28.02	-1.91	155.88	-10.64
305 K	-0.000057	-0.000014		-108.90	-27.13	-24.32	-6.06	29.13	7.26	162.08	40.39
305 R	-0.000051	0.000015		-97.09	27.81	-21.69	6.21	25.97	-7.44	144.50	-41.40

		DISTRIBUTION OF SHEAR FORCES									
		X				Y					
		$E_x + E_{yAD}$		$E_x - E_{yAD}$		$E_y + E_{xAD}$		$E_y - E_{xAD}$			
		E = -62		E = -353		E = 349		E = -109			
	X	Y									
304 F				50.69	124.77	39.57	114.12	32.54	107.39	15.06	90.65
304 B				717.89	41.87	563.78	38.24	466.36	35.95	224.09	30.24
304 H				29.21	285.02	24.44	381.65	21.43	442.72	13.94	594.62
304 J				778.32	49.86	636.58	56.53	546.99	60.75	324.18	71.24
304 K				814.28	59.26	665.99	61.94	572.26	63.64	339.15	67.85
304 R				39.17	529.23	33.52	588.72	29.95	626.33	21.07	719.85
304 S				39.17	571.18	33.52	598.09	29.95	615.11	21.07	657.42
304 T				45.76	561.61	43.23	513.67	41.63	483.38	37.65	408.03
304 U				550.11	55.20	492.81	51.19	456.59	48.65	366.51	42.35
305 F				514.05	55.20	484.76	51.19	466.24	48.65	420.19	42.35
305 B				425.32	514.30	466.85	548.56	493.09	570.21	558.37	624.08
305 H				388.26	549.65	458.57	556.45	503.01	560.76	613.53	571.46
305 J				374.06	565.56	455.39	560.01	506.81	556.50	634.67	547.78
305 K				369.89	531.28	454.46	552.35	507.92	565.67	640.87	598.80
305 R				191.61	364.52	267.01	342.92	314.67	329.27	433.20	295.31

TECHNICAL REPORT 3

APPENDIX E

LEVEL 3				
SHEAR FORCE				
V = 4426				
	X	Y	DISTRIBUTION OF SHEAR FORCES	
			X	Y
304 F	0.005574	0.095647	24.67	423.29
304 G	0.102807	0.006796	454.98	30.08
304 H	0.004566	0.074807	20.21	331.06
304 J	0.117922	0.010679	521.87	47.26
304 K	0.123369	0.011458	545.97	50.71
304 R	0.006312	0.110689	27.93	489.86
304 S	0.006312	0.110689	27.93	489.86
304 T	0.008411	0.091332	37.22	404.19
304 U	0.094274	0.009142	417.21	40.46
305 F	0.094274	0.009142	417.21	40.46
305 G	0.094760	0.102025	419.36	451.51
305 H	0.094760	0.102025	419.36	451.51
305 J	0.094760	0.102025	419.36	451.51
305 K	0.094760	0.102025	419.36	451.51
305 R	0.057138	0.061519	252.87	272.25
TOTAL	1.00000	1.00000		

TORSIONAL SHEAR FORCE					DISTRIBUTION OF SHEAR FORCES								
	X	Y	V _T	X				Y					
				E _X +E _{XADJ}		E _X -E _{XADJ}		E _Y +E _{YADJ}		E _Y -E _{YADJ}			
				E =		E =		E =		E =			
				1525409		234924		-203402.8335		-2232066.329			
304 F	0.000005	0.000035		8.34	52.66	1.28	8.11	-1.11	-7.02	-12.20	-77.05		
304 G	0.000098	0.000002		148.87	3.81	22.93	0.59	-19.85	-0.51	-217.84	-5.57		
304 H	0.000003	-0.000066		4.43	-101.13	0.68	-15.57	-0.59	13.49	-6.49	147.98		
304 J	0.000088	-0.000005		134.07	-6.98	20.65	-1.07	-17.88	0.93	-196.18	10.21		
304 K	0.000092	-0.000002		140.26	-2.80	21.60	-0.43	-18.70	0.37	-205.24	4.09		
304 R	0.000003	-0.000041		5.15	-62.21	0.79	-9.58	-0.69	8.30	-7.53	91.03		
304 S	0.000003	-0.000018		5.15	-28.09	0.79	-4.33	-0.69	3.75	-7.53	41.10		
304 T	0.000001	0.000033		1.62	50.28	0.25	7.74	-0.22	-6.70	-2.38	-73.57		
304 U	0.000032	0.000003		48.52	4.21	7.47	0.65	-6.47	-0.56	-71.00	-6.16		
305 F	0.000013	0.000003		19.19	4.21	2.95	0.65	-2.56	-0.56	-28.08	-6.16		
305 G	-0.000036	-0.000023		-55.04	-35.79	-8.48	-5.51	7.34	4.77	80.54	52.37		
305 H	-0.000056	-0.000005		-85.19	-7.04	-13.12	-1.08	11.36	0.94	124.65	10.30		
305 J	-0.000063	0.000004		-96.74	5.90	-14.90	0.91	12.90	-0.79	141.55	-8.64		
305 K	-0.000066	-0.000014		-100.13	-21.98	-15.42	-3.38	13.35	2.93	146.51	32.16		
305 R	-0.000056	0.000015		-85.93	22.68	-13.23	3.49	11.46	-3.02	125.74	-33.18		

DISTRIBUTION OF SHEAR FORCES										
	X	Y	X				Y			
			E _X +E _{XADJ}		E _X -E _{XADJ}		E _Y +E _{YADJ}		E _Y -E _{YADJ}	
			E =		E =		E =		E =	
			-62		-353		349		-109	
304 F			33.00	475.95	25.95	431.40	23.55	416.27	12.47	346.24
304 G			603.85	33.88	477.90	30.66	435.13	29.57	237.14	24.50
304 H			24.64	229.93	20.89	315.49	19.62	344.55	13.72	479.04
304 J			655.93	40.28	542.51	46.19	503.99	48.19	325.69	57.47
304 K			686.23	47.91	567.57	50.28	527.27	51.08	340.74	54.80
304 R			33.08	427.65	28.73	480.28	27.25	498.15	20.40	580.89
304 S			33.08	461.77	28.73	485.53	27.25	493.60	20.40	530.96
304 T			38.85	454.47	37.47	411.94	37.01	397.49	34.85	330.62
304 U			465.73	44.67	424.68	41.11	410.74	39.90	346.22	34.30
305 F			436.40	44.67	420.17	41.11	414.65	39.90	389.14	34.30
305 G			364.32	415.72	410.89	446.00	426.70	456.29	499.91	503.89
305 H			334.18	444.48	406.24	450.43	430.72	452.45	544.01	461.81
305 J			322.63	457.42	404.47	452.42	432.26	450.73	560.92	442.88
305 K			319.24	429.53	403.94	448.13	432.72	454.45	565.88	483.68
305 R			166.93	294.93	239.63	275.75	264.33	269.23	378.61	239.07

TECHNICAL REPORT 3

APPENDIX E

LEVEL 4

SHEAR FORCE				
V = 3589				
	X	Y	DISTRIBUTION OF SHEAR FORCES	
			X	Y
304 F	0.005574	0.095647	20.01	343.31
304 G	0.102807	0.006796	369.02	24.39
304 H	0.004566	0.074807	16.39	268.51
304 J	0.117922	0.010679	423.27	38.33
304 K	0.123369	0.011458	442.82	41.13
304 R	0.006312	0.110689	22.66	397.31
304 S	0.006312	0.110689	22.66	397.31
304 T	0.008411	0.091332	30.19	327.83
304 U	0.094274	0.009142	338.38	32.81
305 F	0.094274	0.009142	338.38	32.81
305 G	0.094760	0.102025	340.13	366.21
305 H	0.094760	0.102025	340.13	366.21
305 J	0.094760	0.102025	340.13	366.21
305 K	0.094760	0.102025	340.13	366.21
305 R	0.057138	0.061519	205.09	220.81
TOTAL	1.00000	1.00000		

TORSIONAL SHEAR FORCE			DISTRIBUTION OF SHEAR FORCES									
	X	Y	V _T =	X				Y				
				E _X +E _{XADI}		E _X *E _{XADI}		E _Y +E _{YADI}		E _Y *E _{YADI}		
				E =		E =		E =		E =		
				1236779		190116		-169901.0555		-1815273.484		
304 F	0.000005	0.000035		6.76	42.69	1.04	6.56	-0.93	-5.86	-9.92	-62.66	
304 G	0.000098	0.000002		120.70	3.09	18.55	0.47	-16.58	-0.42	-177.16	-4.53	
304 H	0.000003	-0.000066		3.59	-82.00	0.55	-12.60	-0.49	11.26	-5.28	120.35	
304 J	0.000088	-0.000005		108.70	-5.66	16.71	-0.87	-14.93	0.78	-159.54	8.31	
304 K	0.000092	-0.000002		113.72	-2.27	17.48	-0.35	-15.62	0.31	-166.91	3.33	
304 R	0.000003	-0.000041		4.17	-50.44	0.64	-7.75	-0.57	6.93	-6.13	74.03	
304 S	0.000003	-0.000018		4.17	-22.78	0.64	-3.50	-0.57	3.13	-6.13	33.43	
304 T	0.000001	0.000033		1.32	40.77	0.20	6.27	-0.18	-5.60	-1.93	-59.84	
304 U	0.000032	0.000003		39.34	3.41	6.05	0.52	-5.40	-0.47	-57.74	-5.01	
305 F	0.000013	0.000003		15.56	3.41	2.39	0.52	-2.14	-0.47	-22.83	-5.01	
305 G	-0.000036	-0.000023		-44.63	-29.02	-6.86	-4.46	6.13	3.99	65.50	42.59	
305 H	-0.000056	-0.000005		-69.07	-5.71	-10.62	-0.88	9.49	0.78	101.37	8.38	
305 J	-0.000063	0.000004		-78.43	4.79	-12.06	0.74	10.77	-0.66	115.12	-7.03	
305 K	-0.000066	-0.000014		-81.18	-17.82	-12.48	-2.74	11.15	2.45	119.15	26.16	
305 R	-0.000056	0.000015		-69.67	18.39	-10.71	2.83	9.57	-2.53	102.26	-26.99	

			DISTRIBUTION OF SHEAR FORCES									
	X	Y	V _T =	X				Y				
				E _X +E _{XADI}		E _X *E _{XADI}		E _Y +E _{YADI}		E _Y *E _{YADI}		
				E =		E =		E =		E =		
				-62		-353		349		-109		
304 F				26.77	386.01	21.04	349.88	19.08	337.45	10.08	280.65	
304 G				489.72	27.48	387.57	24.87	352.43	23.97	191.86	19.86	
304 H				19.98	186.51	16.94	255.91	15.90	279.77	11.11	388.86	
304 J				531.97	32.67	439.98	37.46	408.33	39.11	263.72	46.64	
304 K				556.54	38.86	460.30	40.78	427.20	41.44	275.90	44.46	
304 R				26.83	346.87	23.30	389.55	22.08	404.23	16.53	471.34	
304 S				26.83	374.53	23.30	393.80	22.08	400.43	16.53	430.73	
304 T				31.51	368.59	30.39	334.09	30.01	322.22	28.26	267.99	
304 U				377.72	36.23	344.43	33.34	332.98	32.35	280.65	27.80	
305 F				353.94	36.23	340.78	33.34	336.25	32.35	315.55	27.80	
305 G				295.50	337.19	333.27	361.75	346.26	370.19	405.63	408.80	
305 H				271.06	360.50	329.51	365.33	349.62	366.99	441.50	374.58	
305 J				261.70	370.99	328.07	366.94	350.91	365.55	455.25	359.18	
305 K				258.95	348.39	327.65	363.47	351.28	368.65	459.28	392.36	
305 R				135.42	239.20	194.38	223.64	214.66	218.29	307.35	193.83	

TECHNICAL REPORT 3

APPENDIX E

LEVEL 5				
SHEAR FORCE				
V = 2503				
	X	Y	DISTRIBUTION OF SHEAR FORCES	
	X	Y	X	Y
304 F	0.001961	0.099319	4.91	248.56
304 G	0.107098	0.002490	268.03	6.23
304 H	0.001280	0.046743	3.20	116.98
304 J	0.123115	0.004109	308.12	10.28
304 K	0.128517	0.004273	321.64	10.69
304 R	0.002221	0.120071	5.56	300.50
304 S	0.002221	0.120071	5.56	300.50
304 T	0.002842	0.072572	7.11	181.62
304 U	0.090242	0.012772	225.85	31.96
305 F	0.090242	0.012772	225.85	31.96
305 G	0.100971	0.113203	252.70	283.31
305 H	0.100971	0.113203	252.70	283.31
305 J	0.100971	0.113203	252.70	283.31
305 K	0.100971	0.113203	252.70	283.31
305 R	0.046379	0.051997	116.07	130.13
TOTAL	1.00000	1.00000		

TORSIONAL SHEAR FORCE			DISTRIBUTION OF SHEAR FORCES								
			X				Y				
			$E_x + E_{xADI}$		$E_x \cdot E_{xADI}$		$E_y + E_{yADI}$		$E_y \cdot E_{yADI}$		
			E =	E =	E =	E =	E =	E =	E =		
			302.8616	11.261596	-254.95845	-713.358					
			757967	28184	-638080.1693	-1785310.033					
	X	Y	V_T								
304 F	0.000002	0.00		1.58	27.82	0.06	1.03	-1.33	-23.42	-3.72	-65.52
304 G	0.000110	0.000001		83.62	0.71	3.11	0.03	-70.40	-0.60	-196.97	-1.67
304 H	0.000001	-0.000041		0.68	-31.28	0.03	-1.16	-0.57	26.33	-1.61	73.67
304 J	0.000101	-0.000002		76.23	-1.32	2.83	-0.05	-64.17	1.11	-179.55	3.10
304 K	0.000105	-0.000001		79.57	-0.50	2.96	-0.02	-66.99	0.42	-187.43	1.17
304 R	0.000001	-0.000044		1.00	-33.02	0.04	-1.23	-0.85	27.80	-2.37	77.79
304 S	0.000001	-0.000019		1.00	-14.56	0.04	-0.54	-0.85	12.26	-2.37	34.29
304 T	0.000000	0.000027		0.37	20.33	0.01	0.76	-0.31	-17.11	-0.86	-47.87
304 U	0.000035	0.000004		26.70	3.00	0.99	0.11	-22.48	-2.53	-62.89	-7.08
305 F	0.000016	0.000004		12.11	3.00	0.45	0.11	-10.20	-2.53	-28.53	-7.08
305 G	-0.000036	-0.000025		-27.60	-19.21	-1.03	-0.71	23.23	16.17	65.00	45.24
305 H	-0.000058	-0.000004		-44.29	-3.29	-1.65	-0.12	37.28	2.77	104.31	7.74
305 J	-0.000067	0.000005		-50.68	3.88	-1.88	0.14	42.67	-3.26	119.38	-9.13
305 K	-0.000069	-0.000015		-52.56	-11.56	-1.95	-0.43	44.24	9.73	123.79	27.23
305 R	-0.000046	0.000013		-34.92	9.84	-1.30	0.37	29.40	-8.29	82.25	-23.18

			DISTRIBUTION OF SHEAR FORCES								
			X				Y				
			$E_x + E_{xADI}$		$E_x \cdot E_{xADI}$		$E_y + E_{yADI}$		$E_y \cdot E_{yADI}$		
			E =	E =	E =	E =	E =	E =	E =		
			-62	-353	349	-109					
	X	Y									
304 F				6.49	276.38	4.97	249.60	3.58	225.15	1.19	183.05
304 G				351.66	6.94	271.14	6.26	197.63	5.63	71.06	4.56
304 H				3.88	85.70	3.23	115.82	2.63	143.31	1.60	190.65
304 J				384.35	8.96	310.95	10.23	243.94	11.39	128.57	13.39
304 K				401.21	10.20	324.60	10.67	254.65	11.11	134.21	11.87
304 R				6.56	267.48	5.60	299.27	4.71	328.30	3.19	378.29
304 S				6.56	285.94	5.60	299.96	4.71	312.76	3.19	334.79
304 T				7.48	201.95	7.13	182.38	6.80	164.51	6.25	133.75
304 U				252.55	34.97	226.84	32.08	203.37	29.43	162.96	24.89
305 F				237.96	34.97	226.30	32.08	215.65	29.43	197.32	24.89
305 G				225.10	264.10	251.67	282.60	275.93	299.48	317.70	328.55
305 H				208.41	280.02	251.05	283.19	289.98	286.08	357.01	291.05
305 J				202.02	287.19	250.81	283.45	295.36	280.05	372.07	274.18
305 K				200.14	271.75	250.74	282.88	296.94	293.04	376.49	310.54
305 R				81.15	139.98	114.77	130.50	145.47	121.85	198.32	106.95

APPENDIX e

LEVEL ML

SHEAR FORCE						
V = 891						
			DISTRIBUTION OF SHEAR FORCES			
		X	Y	X	Y	
BF	305M	0.217197	0.000000	193.45	0.00	
BF	305B	0.000000	0.433016	0.00	385.68	
BF	305T	0.105258	0.000000	93.75	0.00	
BF	305U	0.070658	0.035564	62.93	31.68	
BF	304T_B	0.000000	0.531420	0.00	473.32	
BF	304U_B	0.321644	0.000000	286.48	0.00	
BF	305F_B	0.285243	0.000000	254.06	0.00	
TOTAL		1.00000	1.00000			

TORSIONAL SHEAR FORCE				DISTRIBUTION OF SHEAR FORCES							
				X				Y			
				$E_x + E_{xADI}$		$E_y + E_{yADI}$		$E_x + E_{xADI}$		$E_y + E_{yADI}$	
				$\epsilon = -1.284428$		$\epsilon = -292.88443$		$\epsilon = 52.7570357$		$\epsilon = -405.643$	
		X	Y	V _r =							
BF	305M	0.002331	0.000000	-2.67	0.00	-607.97	0.00	109.51	0.00	-842.04	0.00
BF	305B	0.000000	0.003861	0.00	-4.42	0.00	-1007.22	0.00	181.43	0.00	-1394.99
BF	305T	0.001764	0.000000	-2.02	0.00	-460.10	0.00	82.88	0.00	-637.23	0.00
BF	305U	0.000698	0.000317	-0.80	-0.36	-182.20	-82.72	32.82	14.90	-252.34	-114.57
BF	304T_B	0.000000	0.004178	0.00	-4.78	0.00	-1089.94	0.00	196.33	0.00	-1509.56
BF	304U_B	0.000969	0.000000	-1.11	0.00	-252.81	0.00	45.54	0.00	-350.14	0.00
BF	305F_B	0.001101	0.000000	-1.26	0.00	-287.13	0.00	51.72	0.00	-397.67	0.00

				DISTRIBUTION OF SHEAR FORCES							
				X				Y			
				$E_x + E_{xADI}$		$E_y + E_{yADI}$		$E_x + E_{xADI}$		$E_y + E_{yADI}$	
				$\epsilon = -62$		$\epsilon = -353$		$\epsilon = 349$		$\epsilon = -109$	
BF	305M			190.79	0.00	-414.52	0.00	302.97	0.00	-648.59	0.00
BF	305B			0.00	381.26	0.00	-621.54	0.00	567.11	0.00	-1009.31
BF	305T			91.73	0.00	-366.35	0.00	176.63	0.00	-543.48	0.00
BF	305U			62.13	31.31	-119.26	-51.05	95.75	46.58	-189.41	-82.89
BF	304T_B			0.00	468.54	0.00	-616.62	0.00	669.65	0.00	-1036.24
BF	304U_B			285.37	0.00	33.68	0.00	332.02	0.00	-63.65	0.00
BF	305F_B			252.80	0.00	-33.07	0.00	305.78	0.00	-143.61	0.00

APPENDIX E

LEVEL		RF	
SHEAR FORCE			
V =		733	
		DISTRIBUTION OF SHEAR FORCES	
		X	Y
BF 304F	0.000000	0.071836	
BF 304G	0.091480	0.000000	
BF 304H	0.000000	0.131354	
BF 304J	0.055044	0.000000	
BF 304K	0.043878	0.018974	
BF 304R	0.000000	0.061420	
BF 304S	0.000000	0.191006	
BF 304T	0.000000	0.302929	
BF 304U	0.270082	0.000000	
BF 305F	0.266791	0.000000	
BF 304T_A	0.000000	0.222482	
BF 304U_A	0.144747	0.000000	
BF 305F_A	0.127978	0.000000	
TOTAL	1.00000	1.00000	

TORSIONAL SHEAR FORCE		DISTRIBUTION OF SHEAR FORCES									
		X		Y							
		$E_x + E_{XADI}$	$E_x - E_{XADI}$	$E_y + E_{YADI}$	$E_y - E_{YADI}$						
		$E =$	$E =$	$E =$	$E =$						
		96.93209	-194.66791	-122.82976	-581.23						
V _T =		71045	-142678	-90025.72602	-426001.2573						
		X	Y								
BF 304F	0.000000	0.00052		0.00	36.85	0.00	-74.01	0.00	-46.70	0.00	-220.97
BF 304G	0.001665	0.000000		118.32	0.00	-237.63	0.00	-149.94	0.00	-709.50	0.00
BF 304H	0.000000	0.002712		0.00	192.66	0.00	-386.92	0.00	-244.14	0.00	-1155.25
BF 304J	0.000662	0.000000		47.02	0.00	-94.43	0.00	-59.58	0.00	-281.94	0.00
BF 304K	0.000528	0.000172		37.48	12.23	-75.27	-24.57	-47.50	-15.50	-224.75	-73.36
BF 304R	0.000000	0.000557		0.00	39.60	0.00	-79.53	0.00	-50.18	0.00	-237.46
BF 304S	0.000000	0.000871		0.00	61.89	0.00	-124.29	0.00	-78.42	0.00	-371.09
BF 304T	0.000000	0.002187		0.00	155.40	0.00	-312.09	0.00	-196.92	0.00	-931.83
BF 304U	0.000092	0.000000		6.54	0.00	-13.13	0.00	-8.28	0.00	-39.20	0.00
BF 305F	0.001879	0.000000		133.48	0.00	-268.06	0.00	-169.14	0.00	-800.37	0.00
BF 304T_A	0.000000	0.001606		0.00	114.13	0.00	-229.21	0.00	-144.63	0.00	-684.37
BF 304U_A	0.000049	0.000000		3.50	0.00	-7.04	0.00	-4.44	0.00	-21.01	0.00
BF 305F_A	0.000835	0.000000		59.31	0.00	-119.11	0.00	-75.15	0.00	-355.62	0.00

		DISTRIBUTION OF SHEAR FORCES						
		X		Y				
		$E_x + E_{XADI}$	$E_x - E_{XADI}$	$E_y + E_{YADI}$	$E_y - E_{YADI}$			
		$E =$	$E =$	$E =$	$E =$			
		-62	-353	349	-109			
V _T =		0	0	0	0			
		X	Y					
BF 304F	0.00	89.50	0.00	-21.36	0.00	5.95	0.00	-168.32
BF 304G	185.37	0.00	-170.58	0.00	-82.89	0.00	-642.45	0.00
BF 304H	0.00	288.94	0.00	-290.65	0.00	-147.86	0.00	-1058.98
BF 304J	87.36	0.00	-54.09	0.00	-19.24	0.00	-241.60	0.00
BF 304K	69.64	26.14	-43.11	-10.66	-15.34	-1.60	-192.59	-59.45
BF 304R	0.00	84.62	0.00	-34.52	0.00	-5.17	0.00	-192.45
BF 304S	0.00	201.88	0.00	15.71	0.00	61.57	0.00	-231.10
BF 304T	0.00	377.43	0.00	-90.07	0.00	25.11	0.00	-709.80
BF 304U	204.49	0.00	184.82	0.00	189.67	0.00	158.76	0.00
BF 305F	329.02	0.00	-72.52	0.00	26.40	0.00	-604.83	0.00
BF 304T_A	0.00	277.20	0.00	-66.15	0.00	18.44	0.00	-521.30
BF 304U_A	109.59	0.00	99.05	0.00	101.65	0.00	85.08	0.00
BF 305F_A	153.11	0.00	-25.31	0.00	18.65	0.00	-261.82	0.00

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APPENDIX E

LEVEL		PH	
SHEAR FORCE			
V =		231	
		DISTRIBUTION OF SHEAR FORCES	
		X	Y
BF 306B		0.000000	0.019299
BF 306C1		0.118175	0.000000
BF 306D		0.000000	0.101229
BF 306C2		0.118175	0.000000
BF 306A		0.000000	0.159595
BF 306E		0.208389	0.000000
BF 306F		0.234776	0.000000
BF 306B		0.000000	0.112594
BF 305Q		0.000000	0.221014
BF 306H		0.097270	0.000000
BF 306J		0.000000	0.386269
BF 306K		0.223215	0.000000
TOTAL		1.00000	1.00000

TORSIONAL SHEAR FORCE				DISTRIBUTION OF SHEAR FORCES							
				X				Y			
				$E_x + E_{XADI}$		$E_x - E_{XADI}$		$E_y + E_{YADI}$		$E_y - E_{YADI}$	
				$E =$	$E =$	$E =$	$E =$	$E =$	$E =$	$E =$	$E =$
				-14212	-62	-81452	-353	80483,1917	349	-25218.63534	-109
V_t =				X	Y						
BF 306B		0.000000	0.000255	0.00	-3.62	0.00	-20.76	0.00	20.51	0.00	-6.43
BF 306C1		0.000791	0.000000	-11.25	0.00	-64.46	0.00	63.69	0.00	-19.96	0.00
BF 306D		0.000000	0.000744	0.00	-10.58	0.00	-60.63	0.00	59.91	0.00	-18.77
BF 306C2		0.000464	0.000000	-6.60	0.00	-37.82	0.00	37.37	0.00	-11.71	0.00
BF 306A		0.000000	0.001173	0.00	-16.68	0.00	-95.58	0.00	94.45	0.00	-29.59
BF 306E		0.001129	0.000000	-16.05	0.00	-91.99	0.00	90.89	0.00	-28.48	0.00
BF 306F		0.002115	0.000000	-30.05	0.00	-172.24	0.00	170.19	0.00	-53.33	0.00
BF 306B		0.000000	0.001487	0.00	-21.13	0.00	-121.12	0.00	119.68	0.00	-37.50
BF 305Q		0.000000	0.004488	0.00	-63.78	0.00	-365.53	0.00	361.18	0.00	-113.17
BF 306H		0.001022	0.000000	-14.53	0.00	-83.28	0.00	82.29	0.00	-25.78	0.00
BF 306J		0.000000	0.000828	0.00	-11.77	0.00	-67.44	0.00	66.64	0.00	-20.88
BF 306K		0.000966	0.000000	-13.73	0.00	-78.68	0.00	77.74	0.00	-24.36	0.00

DISTRIBUTION OF SHEAR FORCES											
				X				Y			
				$E_x + E_{XADI}$		$E_x - E_{XADI}$		$E_y + E_{YADI}$		$E_y - E_{YADI}$	
				$E =$	$E =$	$E =$	$E =$	$E =$	$E =$	$E =$	$E =$
				0	-62	0	-353	0	349	0	-109
BF 306B				0.00	0.83	0.00	-16.31	0.00	24.96	0.00	-1.98
BF 306C1				16.00	0.00	-64.46	0.00	63.69	0.00	-19.96	0.00
BF 306D				0.00	12.76	0.00	-60.63	0.00	59.91	0.00	-18.77
BF 306C2				20.65	0.00	-37.82	0.00	37.37	0.00	-11.71	0.00
BF 306A				0.00	20.12	0.00	-95.58	0.00	94.45	0.00	-29.59
BF 306E				32.00	0.00	-91.99	0.00	90.89	0.00	-28.48	0.00
BF 306F				24.08	0.00	-172.24	0.00	170.19	0.00	-53.33	0.00
BF 306B				0.00	4.83	0.00	-121.12	0.00	119.68	0.00	-37.50
BF 305Q				0.00	-12.82	0.00	-365.53	0.00	361.18	0.00	-113.17
BF 306H				7.90	0.00	-83.28	0.00	82.29	0.00	-25.78	0.00
BF 306J				0.00	77.30	0.00	-67.44	0.00	66.64	0.00	-20.88
BF 306K				37.74	0.00	-78.68	0.00	77.74	0.00	-24.36	0.00

APPENDIX 9

LEVEL		LOAD APP: 0					
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	21558	0	21558	-6987	-44870	-37883
	(-)	44904	0	44904	-2952	-18957	-16005
EAST	(+)	0	-25978	25978	-4490	-28834	-24344
	(-)	0	-10817	10817	-1870	-12006	-10136
SOUTH	(+)	2297	-33763	36061	-30419	0	30419
	(-)	5366	-14058	19424	-12666	0	12666
NORTH	(+)	14752	-5258	20010	-30419	0	30419
	(-)	34457	-2189	36647	-12666	0	12666

LEVEL		LOAD APP: 0					
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE		ECCENTRICITY	
		Fs X	Fs Y	X	Y	Ex	Ey
WEST	(+)	2945250	0	136.62	0	-76.6687	0
	(-)	6134802	0	136.62	0	-76.6687	0
EAST	(+)	#N/A	#N/A	0	0	0	0
	(-)	#N/A	#N/A	0	0	0	0
SOUTH	(+)	0	-80014	0	-34.83	0	-129.867
	(-)	0	-186890	0	-34.83	0	-129.867
NORTH	(+)	0	420568	0	28.5084	0	-66.5285
	(-)	0	982325	0	28.5084	0	-66.5285

C.O.R.

X = 2559.46 Y = 1140.44

LEEWARD		RESULTANT COORDINATE		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	0	0	0	0	0
0	0	0	0	0	0
-3549070	0	136.62	0	-76.6687	0
-1477762	0	136.62	0	-76.6687	0
0	-962538	0	28.5084	0	-66.5285
0	-400782	0	28.5084	0	-66.5285
0	183125	0	-34.83	0	-129.867
0	76250	0	-34.83	0	-129.867

SIDE 1		RESULTANT COORDINATE		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	243368	0	-34.83	0	-129.867
0	102820	0	-34.83	0	-129.867
0	156391	0	-34.83	0	-129.867
0	65118	0	-34.83	0	-129.867
-4155783	0	136.62	0	-76.6687	0
-1730386	0	136.62	0	-76.6687	0
-4155783	0	136.62	0	-76.6687	0
-1730386	0	136.62	0	-76.6687	0

SIDE 2		RESULTANT COORDINATE		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	-1279183	0	28.5084	0	-66.5285
0	-540438	0	28.5084	0	-66.5285
0	-822015	0	28.5084	0	-66.5285
0	-342271	0	28.5084	0	-66.5285
#N/A	#N/A	0	0	0	0
#N/A	#N/A	0	0	0	0
#N/A	#N/A	0	0	0	0
#N/A	#N/A	0	0	0	0

LEVEL		[=] FT LB										
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2		TORSION		
		F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	Fex	Fey	TORSION
WEST	(+)	-1652825	0	0	0	0	907421	0	2985156	-1652825	2077735	-3730561
	(-)	-3442750	0	0	0	0	383373	0	1261189	-3442750	877815	-4320565
EAST	(+)	0	0	1991680	0	0	583117	0	1918289	1991680	1335172	656508
	(-)	0	0	829296	0	0	242798	0	798737	829296	555939	273357
SOUTH	(+)	0	-298341	0	2246220	2332157	0	0	0	2332157	2544561	-212404
	(-)	0	-696837	0	935281	971064	0	0	0	971064	1632118	-661054
NORTH	(+)	0	-981456	0	682801	2332157	0	0	0	2332157	-1664257	3996414
	(-)	0	-2292395	0	284305	971064	0	0	0	971064	-2576699	3547763

APPENDIX 9

LEVEL 1		LOAD APP: 1					
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	23568	-25080	48648	-55659	-117646	-61987
	(-)	47925	-10596	58521	-23515	-49704	-26189
EAST	(+)	9050	-27103	36153	-35767	-75600	-39833
	(-)	18456	-11285	29741	-14893	-31479	-16586
SOUTH	(+)	19152	-88524	107677	-31736	-18872	12864
	(-)	43595	-36860	80455	-13214	-7858	5356
NORTH	(+)	40482	-41882	82364	-31736	-18872	12864
	(-)	92147	-17439	109585	-13214	-7858	5356

LEVEL 1		LOAD APP: 1					
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE:		ECCENTRICITY	
		F _s X	F _s Y	X	Y	Ex	Ey
WEST	(+)	3366347	0	142.836	0	-70.4523	0
	(-)	6847138	0	142.871	0	-70.4175	0
EAST	(+)	1144805	0	126.5	0	-86.7885	0
	(-)	2334674	0	126.5	0	-86.7885	0
SOUTH	(+)	0	589396	0	30.7743	0	-64.2627
	(-)	0	1323126	0	30.3503	0	-64.6867
NORTH	(+)	0	5710247	0	141.056	0	46.0192
	(-)	0	#####	0	141.947	0	46.9102

C.O.R. X = 2559.46 Y = 1140.44

LEEWARD		RESULTANT COORDINATE:		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
-3172650	0	126.5	0	-86.7885	0
-1340402	0	126.5	0	-86.7885	0
-3873137	0	142.905	0	-70.3839	0
-1612698	0	142.905	0	-70.3839	0
0	#####	0	142.645	0	47.6083
0	-5257873	0	142.645	0	47.6083
0	-1257205	0	30.018	0	-65.0189
0	-523475	0	30.018	0	-65.0189

SIDE 1		RESULTANT COORDINATE:		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	-1670787	0	30.018	0	-65.0189
0	-705885	0	30.018	0	-65.0189
0	-1073663	0	30.018	0	-65.0189
0	-447052	0	30.018	0	-65.0189
-4535249	0	142.905	0	-70.3839	0
-1888388	0	142.905	0	-70.3839	0
-4535249	0	142.905	0	-70.3839	0
-1888388	0	142.905	0	-70.3839	0

SIDE 2		RESULTANT COORDINATE:		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	#####	0	142.645	0	47.6083
0	-7090026	0	142.645	0	47.6083
0	#####	0	142.645	0	47.6083
0	-4490263	0	142.645	0	47.6083
-2387302	0	126.5	0	-86.7885	0
-994025	0	126.5	0	-86.7885	0
-2387302	0	126.5	0	-86.7885	0
-994025	0	126.5	0	-86.7885	0

LEVEL 1		[=] FT LB										
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2		FEX	FEY	TORSION
		F(X)	F(Y)	F(X)	F(Y)	F(X)	F(Y)	F(X)	F(Y)			
WEST	(+)	-1660412	0	2176676	0	0	3618914	0	-5600929	-3837088	-9219843	5382755
	(-)	-3374784	0	919617	0	0	1528943	0	-2366318	-4294401	-3895260	-399140
EAST	(+)	-785422	0	1907612	0	0	2325547	0	-3599209	2693034	-5924756	8617790
	(-)	-1601761	0	794292	0	0	968312	0	-1498639	2396053	-2466951	4863004
SOUTH	(+)	0	-1230774	0	-4214492	2233718	0	1637869	0	595849	-2983719	3579568
	(-)	0	-2820028	0	-1754831	930076	0	681976	0	248100	1065197	-817097
NORTH	(+)	0	1862954	0	2723099	2233718	0	1637869	0	595849	-860145	1455994
	(-)	0	4322615	0	1133844	930076	0	681976	0	248100	3188771	-2940671

APPENDIX 9

LEVEL 2		LOAD APP: 2					
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	52863	-66001	118864	-108925	-152791	-43867
	(-)	101080	-27884	128965	-46019	-64552	-18533
EAST	(+)	23815	-53652	77467	-69996	-98185	-28189
	(-)	48568	-22340	70908	-29145	-40882	-11737
SOUTH	(+)	46236	-114970	161206	-62824	-49663	13161
	(-)	94070	-47871	141942	-26159	-20679	5480
NORTH	(+)	64373	-81962	146335	-62824	-49663	13161
	(-)	131472	-34127	165599	-26159	-20679	5480

LEVEL 2		LOAD APP: 2					
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE:		ECCENTRICITY	
		Fs X	Fs Y	X	Y	Ex	Ey
WEST	(+)	7475209	0	141.406	0	-71.882	0
	(-)	#####	0	141.406	0	-71.882	0
EAST	(+)	3689238	0	154.91	0	-58.3785	0
	(-)	7523701	0	154.91	0	-58.3785	0
SOUTH	(+)	0	5013062	0	108.424	0	13.3868
	(-)	0	#####	0	107.797	0	12.7601
NORTH	(+)	0	6885173	0	106.957	0	11.9202
	(-)	0	#####	0	106.957	0	11.9202

C.O.R. X = 2559.46 Y = 1140.44

LEEWARD		RESULTANT COORDINATE:		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
#####	0	154.91	0	-58.3785	0
-4319570	0	154.91	0	-58.3785	0
-7586734	0	141.406	0	-71.882	0
-3158965	0	141.406	0	-71.882	0
0	#####	0	106.957	0	11.9202
0	-5120167	0	106.957	0	11.9202
0	-8785587	0	107.191	0	12.1544
0	-3658144	0	107.191	0	12.1544

SIDE 1		RESULTANT COORDINATE:		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	#####	0	107.191	0	12.1544
0	-4932857	0	107.191	0	12.1544
0	-7502958	0	107.191	0	12.1544
0	-3124083	0	107.191	0	12.1544
-8883684	0	141.406	0	-71.882	0
-3698990	0	141.406	0	-71.882	0
-8883684	0	141.406	0	-71.882	0
-3698990	0	141.406	0	-71.882	0

SIDE 2		RESULTANT COORDINATE:		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	#####	0	106.957	0	11.9202
0	-6904336	0	106.957	0	11.9202
0	#####	0	106.957	0	11.9202
0	-4372662	0	106.957	0	11.9202
-7693301	0	154.91	0	-58.3785	0
-3203338	0	154.91	0	-58.3785	0
-7693301	0	154.91	0	-58.3785	0
-3203338	0	154.91	0	-58.3785	0

LEVEL 2		[=] FT LB										
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2		FEX	FEY	TORSION
		F(EX)	F(EY)	F(EX)	F(EY)	F(EX)	F(EY)	F(EX)	F(EY)			
WEST	(+)	-3799919	0	3853018	0	0	-1323909	0	-1821306	-7652937	-497397	-7155539
	(-)	-7265858	0	1627849	0	0	-559334	0	-769477	-8893707	-210144	-8683563
EAST	(+)	-1390305	0	3856611	0	0	-850756	0	-1170388	5246916	-319632	5566548
	(-)	-2835339	0	1605816	0	0	-354238	0	-487326	4441155	-133088	4574244
SOUTH	(+)	0	618951	0	-1370465	4515898	0	2899254	0	1616644	-1989417	3606061
	(-)	0	1200349	0	-570635	1880330	0	1207192	0	673139	-1770983	2444122
NORTH	(+)	0	767342	0	-996192	4515898	0	2899254	0	1616644	1763534	-146890
	(-)	0	1567172	0	-414795	1880330	0	1207192	0	673139	1981967	-1308829

APPENDIX 9

LEVEL 3		LOAD APP: 3					
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	59454	-79861	139315	-126391	-152791	-26400
	(-)	107671	-33740	141411	-53399	-64552	-11154
EAST	(+)	28817	-53652	82469	-81220	-98185	-16965
	(-)	58768	-22340	81107	-33818	-40882	-7064
SOUTH	(+)	62789	-114970	177759	-62824	-60092	2731
	(-)	118294	-47871	166165	-26159	-25021	1137
NORTH	(+)	75905	-95105	171009	-62824	-60092	2731
	(-)	143003	-39600	182603	-26159	-25021	1137

LEVEL 3		LOAD APP: 3					
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE		ECCENTRICITY	
		F _s X	F _s Y	X	Y	Ex	Ey
WEST	(+)	8407144	0	141.406	0	-71.882	0
	(-)	#####	0	141.406	0	-71.882	0
EAST	(+)	4942045	0	171.5	0	-41.7885	0
	(-)	#####	0	171.5	0	-41.7885	0
SOUTH	(+)	0	8099817	0	129	0	33.9631
	(-)	0	#####	0	129	0	33.9631
NORTH	(+)	0	8118533	0	106.957	0	11.9202
	(-)	0	#####	0	106.957	0	11.9202

C.O.R. X = 2559.46 Y = 1140.44

LEEWARD		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
#####	0	171.5	0	-41.7885	0
-5786428	0	171.5	0	-41.7885	0
-7586734	0	141.406	0	-71.882	0
-3158965	0	141.406	0	-71.882	0
0	#####	0	106.957	0	11.9202
0	-5120167	0	106.957	0	11.9202
0	#####	0	129	0	33.9631
0	-5108364	0	129	0	33.9631

SIDE 1		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	#####	0	129	0	33.9631
0	-6888419	0	129	0	33.9631
0	#####	0	129	0	33.9631
0	-4362581	0	129	0	33.9631
-8883684	0	141.406	0	-71.882	0
-3698990	0	141.406	0	-71.882	0
-8883684	0	141.406	0	-71.882	0
-3698990	0	141.406	0	-71.882	0

SIDE 2		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	#####	0	106.957	0	11.9202
0	-6904336	0	106.957	0	11.9202
0	#####	0	106.957	0	11.9202
0	-4372662	0	106.957	0	11.9202
#####	0	171.5	0	-41.7885	0
-4291141	0	171.5	0	-41.7885	0
#####	0	171.5	0	-41.7885	0
-4291141	0	171.5	0	-41.7885	0

LEVEL 3		[=] FT LB										
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2		F _{EX}	F _{EY}	TORSION
		F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)			
WEST	(+)	-4273655	0	3337262	0	0	-4292632	0	-1821306	-7610917	2471326	-1E+07
	(-)	-7739595	0	1409948	0	0	-1813580	0	-769477	-9149543	1044102	-1E+07
EAST	(+)	-1204202	0	3856611	0	0	-2758485	0	-1170388	5060813	1588097	3472716
	(-)	-2455807	0	1605816	0	0	-1148578	0	-487326	4061623	661252	3400371
SOUTH	(+)	0	2132516	0	-1370465	4515898	0	2511166	0	2004732	-3502981	5507713
	(-)	0	4017635	0	-570635	1880330	0	1045600	0	834730	-4588270	5423000
NORTH	(+)	0	904798	0	-3230047	4515898	0	2511166	0	2004732	4134845	-2130113
	(-)	0	1704629	0	-1344927	1880330	0	1045600	0	834730	3049556	-2214825

APPENDIX 9

LEVEL 4		LOAD APP: 4					
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	64510	-79861	144371	-126391	-152791	-26400
	(-)	112727	-33740	146467	-53399	-64552	-11154
EAST	(+)	32929	-53652	86581	-81220	-98185	-16965
	(-)	62880	-22340	85219	-33818	-40882	-7064
SOUTH	(+)	70298	-114970	185268	-62824	-60092	2731
	(-)	125803	-47871	173674	-26159	-25021	1137
NORTH	(+)	84982	-95105	180086	-62824	-60092	2731
	(-)	152080	-39600	191680	-26159	-25021	1137

LEVEL 4		LOAD APP: 4					
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE		ECCENTRICITY	
		F _s X	F _s Y	X	Y	Ex	Ey
WEST	(+)	9122127	0	141.406	0	-71.882	0
	(-)	#####	0	141.406	0	-71.882	0
EAST	(+)	9391440	0	285.206	0	71.9179	0
	(-)	#####	0	288.681	0	75.392	0
SOUTH	(+)	0	9068430	0	129	0	33.9631
	(-)	0	#####	0	129	0	33.9631
NORTH	(+)	0	9089384	0	106.957	0	11.9202
	(-)	0	#####	0	106.957	0	11.9202

C.O.R. X = 2559.46 Y = 1140.44

LEEWARD		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
#####	0	292.5	0	79.2115	0
-9868981	0	292.5	0	79.2115	0
-7586734	0	141.406	0	-71.882	0
-3158965	0	141.406	0	-71.882	0
0	#####	0	106.957	0	11.9202
0	-5120167	0	106.957	0	11.9202
0	#####	0	129	0	33.9631
0	-5108364	0	129	0	33.9631

SIDE 1		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	#####	0	129	0	33.9631
0	-6888419	0	129	0	33.9631
0	#####	0	129	0	33.9631
0	-4362581	0	129	0	33.9631
-8883684	0	141.406	0	-71.882	0
-3698990	0	141.406	0	-71.882	0
-8883684	0	141.406	0	-71.882	0
-3698990	0	141.406	0	-71.882	0

SIDE 2		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	#####	0	106.957	0	11.9202
0	-6904336	0	106.957	0	11.9202
0	#####	0	106.957	0	11.9202
0	-4372662	0	106.957	0	11.9202
#####	0	292.5	0	79.2115	0
-7318710	0	292.5	0	79.2115	0
#####	0	292.5	0	79.2115	0
-7318710	0	292.5	0	79.2115	0

LEVEL 4		[=] FT LB										
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2				
		F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F _{EX}	F _{EY}	TORSION
WEST	(+)	-4637107	0	-6325891	0	0	-4292632	0	-1821306	1688785	2471326	-782541
	(-)	-8103046	0	-2672604	0	0	-1813580	0	-769477	-5430441	1044102	-6474544
EAST	(+)	2368155	0	3856611	0	0	-2758485	0	-1170388	1488456	1588097	-99640.8
	(-)	4740614	0	1605816	0	0	-1148578	0	-487326	-3134798	661252	-3796050
SOUTH	(+)	0	2387532	0	-1370465	4515898	0	-4759999	0	9275897	-3757997	1.3E+07
	(-)	0	4272651	0	-570635	1880330	0	-1981969	0	3862299	-4843286	8705585
NORTH	(+)	0	1012998	0	-3230047	4515898	0	-4759999	0	9275897	4243044	5032853
	(-)	0	1812828	0	-1344927	1880330	0	-1981969	0	3862299	3157756	704544

APPENDIX 9

LEVEL 5		LOAD APP: 5					
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	78128	-71440	149568	-112813	-134310	-21497
	(-)	133005	-30182	163187	-47662	-56744	-9082
EAST	(+)	35326	-61062	96388	-72495	-86309	-13814
	(-)	62119	-25425	87544	-30185	-35937	-5752
SOUTH	(+)	68598	-101063	169661	-71500	-53756	17744
	(-)	118140	-42081	160221	-29771	-22383	7388
NORTH	(+)	80987	-84887	165874	-71500	-53756	17744
	(-)	139969	-35345	175315	-29771	-22383	7388

LEVEL 5		LOAD APP: 5					
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE		ECCENTRICITY	
		F _s X	F _s Y	X	Y	Ex	Ey
WEST	(+)	#####	0	160.271	0	-53.018	0
	(-)	#####	0	160.177	0	-53.1115	0
EAST	(+)	6058375	0	171.5	0	-41.7885	0
	(-)	#####	0	171.5	0	-41.7885	0
SOUTH	(+)	0	8636786	0	125.905	0	30.8677
	(-)	0	#####	0	126.357	0	31.3199
NORTH	(+)	0	9081705	0	112.138	0	17.1015
	(-)	0	#####	0	112.088	0	17.0514

C.O.R. X = 2559.46 Y = 1140.44

LEEWARD		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
#####	0	171.5	0	-41.7885	0
-5176295	0	171.5	0	-41.7885	0
-9772582	0	160.044	0	-53.2446	0
-4069109	0	160.044	0	-53.2446	0
0	#####	0	112.02	0	16.9827
0	-4713878	0	112.02	0	16.9827
0	#####	0	126.983	0	31.9459
0	-4488267	0	126.983	0	31.9459

SIDE 1		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	#####	0	126.983	0	31.9459
0	-6052244	0	126.983	0	31.9459
0	-9205563	0	126.983	0	31.9459
0	-3833014	0	126.983	0	31.9459
#####	0	160.044	0	-53.2446	0
-4764723	0	160.044	0	-53.2446	0
#####	0	160.044	0	-53.2446	0
-4764723	0	160.044	0	-53.2446	0

SIDE 2		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	#####	0	112.02	0	16.9827
0	-6356471	0	112.02	0	16.9827
0	-9668299	0	112.02	0	16.9827
0	-4025688	0	112.02	0	16.9827
-9219157	0	171.5	0	-41.7885	0
-3838674	0	171.5	0	-41.7885	0
-9219157	0	171.5	0	-41.7885	0
-3838674	0	171.5	0	-41.7885	0

LEVEL 5		[=] FT LB										
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2		F _x	F _y	TORSION
		F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)			
WEST	(+)	-4142210	0	2985373	0	0	-3603905	0	-2280951	-7127584	1322954	-8450538
	(-)	-7064083	0	1261280	0	0	-1522602	0	-963671	-8325364	558930	-8884294
EAST	(+)	-1476212	0	3251219	0	0	-2315902	0	-1465760	4727431	850142	3877289
	(-)	-2595845	0	1353743	0	0	-964296	0	-610313	3949588	353982	3595606
SOUTH	(+)	0	2117459	0	-1716331	3807014	0	2246383	0	1560631	-3833790	5394421
	(-)	0	3700123	0	-714646	1585165	0	935349	0	649816	-4414769	5064585
NORTH	(+)	0	1384991	0	-2711805	3807014	0	2246383	0	1560631	4096796	-2536165
	(-)	0	2386676	0	-1129142	1585165	0	935349	0	649816	3515817	-2866002

APPENDIX 9

LEVEL		ML		LOAD APP: 6			
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	44105	-39250	83355	-6735	-41298	-34563
	(-)	73972	-16583	90555	-2845	-17448	-14602
EAST	(+)	22377	-33233	55610	-4328	-26538	-22210
	(-)	37097	-13838	50935	-1802	-11050	-9248
SOUTH	(+)	4532	-31075	35607	-38915	-29534	9380
	(-)	7489	-12939	20428	-16203	-12297	3906
NORTH	(+)	26439	-5068	31507	-38915	-29534	9380
	(-)	44575	-2110	46685	-16203	-12297	3906

LEVEL		ML		LOAD APP: 6			
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE		ECCENTRICITY	
		F _s X	F _s Y	X	Y	Ex	Ey
WEST	(+)	8507626	0	192.894	0	-20.3949	0
	(-)	#####	0	192.894	0	-20.3947	0
EAST	(+)	4239911	0	189.479	0	-23.8099	0
	(-)	7020146	0	189.237	0	-24.0514	0
SOUTH	(+)	0	856111	0	188.914	0	93.8773
	(-)	0	1410267	0	188.304	0	93.2667
NORTH	(+)	0	5827973	0	220.429	0	125.392
	(-)	0	9822696	0	220.363	0	125.326

C.O.R. X = 2559.46 Y = 1140.44

LEEWARD		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
-7413178	0	188.87	0	-24.4186	0
-3131968	0	188.87	0	-24.4186	0
-6410513	0	192.894	0	-20.3943	0
-2669210	0	192.894	0	-20.3943	0
0	-6844734	0	220.266	0	125.229
0	-2850012	0	220.266	0	125.229
0	-949517	0	187.368	0	92.3311
0	-395360	0	187.368	0	92.3311

SIDE 1		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	-1261878	0	187.368	0	92.3311
0	-533127	0	187.368	0	92.3311
0	-810895	0	187.368	0	92.3311
0	-337640	0	187.368	0	92.3311
-7506389	0	192.894	0	-20.3943	0
-3125511	0	192.894	0	-20.3943	0
-7506389	0	192.894	0	-20.3943	0
-3125511	0	192.894	0	-20.3943	0

SIDE 2		RESULTANT COORDINATE		ECCENTRICITY	
F _s X	F _s Y	X	Y	Ex	Ey
0	-9096439	0	220.266	0	125.229
0	-3843124	0	220.266	0	125.229
0	-5845455	0	220.266	0	125.229
0	-2433932	0	220.266	0	125.229
-5578142	0	188.87	0	-24.4186	0
-2322628	0	188.87	0	-24.4186	0
-5578142	0	188.87	0	-24.4186	0
-2322628	0	188.87	0	-24.4186	0

LEVEL		ML		[=] FT LB								
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2		TORSION		
		F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F _{Ex}	F _{Ey}	TORSION
WEST	(+)	-899524	0	958433	0	0	-621827	0	-5171647	-1857957	-4549820	2691863
	(-)	-1508637	0	404925	0	0	-262714	0	-2184952	-1913562	-1922238	8676.26
EAST	(+)	-532787	0	677770	0	0	-399592	0	-3323348	1210557	-2923756	4134313
	(-)	-892237	0	282210	0	0	-166382	0	-1383776	1174447	-1217394	2391841
SOUTH	(+)	0	425428	0	-3891473	793635	0	721185	0	72449	-4316901	4389351
	(-)	0	698505	0	-1620332	330454	0	300287	0	30167	-2318837	2349004
NORTH	(+)	0	3315268	0	-467902	793635	0	721185	0	72449	3783170	-9710721
	(-)	0	5586409	0	-194825	330454	0	300287	0	30167	5781234	-5751068

APPENDIX 9

Level		M.I.		SOUTH				NORTH					
DIRECT SHEAR		RELATIVE RIGIDITY		(+)		(-)		(+)		(-)			
				Vx	Vy	Vx	Vy	Vx	Vy	Vx	Vy		
				35607	9380	20428	3906	31507	9380	46685	3906		
SHEAR WALL		X-DIRECTION	Y-DIRECTION	X	Y	X	Y	X	Y	X	Y		
BF	305M	0.2171968	0.0000000	7733.64	0	4436.96	0	6843.2	0	10139.9	0		
BF	305S	0.0000000	0.4330162	0	4061.79	0	1691.25	0	4061.79	0	1691.25		
BF	305T	0.1052581	0.0000000	3747.89	0	2150.24	0	3316.36	0	4914	0		
BF	305U	0.0706577	0.0355636	2515.88	333.595	1443.42	138.902	2226.2	333.595	3298.67	138.902		
BF	304T_B	0.0000000	0.5314202	0	4984.84	0	2075.59	0	4984.84	0	2075.59		
BF	304U_B	0.3216444	0.0000000	11452.7	0	6570.64	0	10134	0	15016	0		
BF	305F_B	0.2852430	0.0000000	10156.5	0	5827.03	0	8987.12	0	13316.6	0		
TOTAL		1.0000	1.0000										

Level		M.I.		SOUTH				NORTH					
TORSIONAL SHEAR		RELATIVE RIGIDITY		(+)		(-)		(+)		(-)			
				TORSION = 4389351		TORSION = 2349004		TORSION = -3710721		TORSION = -5751068			
				X	Y	X	Y	X	Y	X	Y		
SHEAR WALL		X-DIRECTION	Y-DIRECTION	X	Y	X	Y	X	Y	X	Y		
BF	305M	0.0023306	0.0000000	10229.8	0	5474.6	0	-8648.22	0	-13403.5	0		
BF	305S	0.0000000	0.0038611	0	16947.6	0	9069.66	0	-14327.3	0	-22205.3		
BF	305T	0.0017637	0.0000000	7741.67	0	4143.03	0	-6544.74	0	-10143.4	0		
BF	305U	0.0006984	0.0003171	3065.64	1391.9	1640.61	744.891	-2591.66	-1176.7	-4016.7	-1823.72		
BF	304T_B	0.0000000	0.0041782	0	18339.5	0	9814.55	0	-15504	0	-24029		
BF	304U_B	0.0009691	0.0000000	4253.75	0	2276.43	0	-3596.08	0	-5573.39	0		
BF	305F_B	0.0011007	0.0000000	4831.22	0	2585.47	0	-4084.27	0	-6330.02	0		
TOTAL		0.0069	0.0084										

Level		M.I.		SOUTH				NORTH			
TOTAL SHEAR				(+)		(-)		(+)		(-)	
				Vx	Vy	Vx	Vy	Vx	Vy	Vx	Vy
				X	Y	X	Y	X	Y	X	Y
SHEAR WALL		X	Y	X	Y	X	Y	X	Y	X	Y
	304 F	17963.5	0	9911.55	0	-1805.02	0	-3263.58	0		
	304 G	0	-12885.8	0	-7378.41	0	18389.1	0	23896.5	0	
	304 H	11489.6	0	6293.27	0	-3228.39	0	-5229.39	0		
	304 J	5581.52	-1058.31	3084.02	-605.989	-365.46	1510.3	-718.027	1962.62		
	304 K	0	-13354.6	0	-7738.96	0	20488.9	0	26104.6		
	304 R	15706.4	0	8847.08	0	6537.94	0	9442.65	0		
	304 S	14987.8	0	8412.5	0	4902.85	0	6986.62	0		
	304 T	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!
	304 U	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!
	305 F	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!
	305 G	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!
	305 H	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!
	305 J	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!
	305 K	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!
	305 R	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!	#REF!
TOTAL											

APPENDIX 9

LEVEL	RF	LOAD APP: 7					
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	53212	-64941	118153	-95656	-68976	26680
	(-)	88047	-27437	115484	-40413	-29141	11272
EAST	(+)	35574	-38761	74335	-61469	-44324	17145
	(-)	59929	-16139	76069	-25595	-18456	7139
SOUTH	(+)	62020	-51902	113922	-45388	-48866	-3478
	(-)	104028	-21611	125639	-18899	-20347	-1448
NORTH	(+)	45413	-71978	117391	-45388	-48866	-3478
	(-)	75704	-29970	105674	-18899	-20347	-1448

LEVEL	RF	LOAD APP: 7					
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE		ECCENTRICITY	
		Fs X	Fs Y	X	Y	Ex	Ey
WEST	(+)	10498308	0	197.291	0	-15.9971	0
	(-)	17369251	0	197.272	0	-16.0162	0
EAST	(+)	6291690	0	176.863	0	-36.4254	0
	(-)	10600678	0	176.887	0	-36.4019	0
SOUTH	(+)	0	12811419	0	206.567	0	111.531
	(-)	0	21478979	0	206.473	0	111.436
NORTH	(+)	0	8811919	0	194.039	0	99.0016
	(-)	0	14651008	0	193.53	0	98.493

C.O.R.

X = 2559.46198 Y = 1140.44

LEEWARD		RESULTANT COORDINATE		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
-11489426	0	176.921	0	-36.3676	0
-4854129	0	176.921	0	-36.3676	0
-7645417	0	197.243	0	-16.0453	0
-3183400	0	197.243	0	-16.0453	0
0	-10004952	0	192.767	0	97.7305
0	-4165863	0	192.767	0	97.7305
0	-14851380	0	206.333	0	111.296
0	-6183820	0	206.333	0	111.296

SIDE 1		RESULTANT COORDINATE		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	-19737023	0	206.333	0	111.296
0	-8338628	0	206.333	0	111.296
0	-12683192	0	206.333	0	111.296
0	-5281029	0	206.333	0	111.296
-8952400	0	197.243	0	-16.0453	0
-3727601	0	197.243	0	-16.0453	0
-8952400	0	197.243	0	-16.0453	0
-3727601	0	197.243	0	-16.0453	0

SIDE 2		RESULTANT COORDINATE		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	-13296270	0	192.767	0	97.7305
0	-5617496	0	192.767	0	97.7305
0	-8544305	0	192.767	0	97.7305
0	-3557679	0	192.767	0	97.7305
-8645369	0	176.921	0	-36.3676	0
-3599760	0	176.921	0	-36.3676	0
-8645369	0	176.921	0	-36.3676	0
-3599760	0	176.921	0	-36.3676	0

LEVEL	RF	[=] FT LB										
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2		TORSION		
		F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	Fex	Fey	TORSION
WEST	(+)	-851239.213	0	2361749	0	0	-1.1E+07	0	-6741028.8	-3212988.1	3905138	-7E+06
	(-)	-1410176	0	997807	0	0	-4497863	0	-2847994.5	-2407983.3	1649868	-4E+06
EAST	(+)	-1295791	0	621939	0	0	-6841324	0	-4331847	1917730	2509477	-591748
	(-)	-2181540	0	258963	0	0	-2848592	0	-1803695.1	2440503	1044897	1395606
SOUTH	(+)	0	6917180	0	-5072375	728259	0	1777129	0	-1048870	#####	1.1E+07
	(-)	0	11592470	0	-2112036	303233	0	739961	0	-436729	#####	1.3E+07
NORTH	(+)	0	4495984	0	-8010847	728259	0	1777129	0	-1048870	#####	-1E+07
	(-)	0	7456323	0	-3335558	303233	0	739961	0	-436729	#####	-1E+07

TECHNICAL REPORT 3

APPENDIX 9

LEVEL		RF		WEST				EAST			
DIRECT SHEAR		RELATIVE RIGIDITY		(+)		(-)		(+)		(-)	
				Vx	Vy	Vx	Vy	Vx	Vy	Vx	Vy
				26680	118153	115484	11272	17145	74335	7139	76069
SHEAR WALL		X-DIRECTION	Y-DIRECTION	X	Y	X	Y	X	Y	X	Y
BF	304F	0.0000000	0.0718356	0	8487.6	0	809.736	0	5339.91	0	5464.44
BF	304G	0.0914799	0.0000000	2440.71	0	10564.5	0	1568.42	0	653.061	0
BF	304H	0.0000000	0.1313538	0	15519.9	0	1480.63	0	9764.21	0	9991.91
BF	304J	0.0550442	0.0000000	1468.6	0	6356.72	0	943.734	0	392.952	0
BF	304K	0.0438783	0.0189744	1170.69	2241.89	5067.23	213.881	752.294	1410.47	313.24	1443.36
BF	304R	0.0000000	0.0614196	0	7256.93	0	692.327	0	4565.64	0	4672.11
BF	304S	0.0000000	0.1910062	0	22568	0	2153.04	0	14198.5	0	14529.6
BF	304T	0.0000000	0.3029286	0	35792	0	3414.63	0	22518.2	0	23043.4
BF	304U	0.2700823	0.0000000	7205.88	0	31190.1	0	4630.56	0	1928.07	0
BF	305F	0.2667906	0.0000000	7118.06	0	30810	0	4574.13	0	1904.58	0
BF	304T_A	0.0000000	0.2224818	0	26286.9	0	2507.83	0	16538.2	0	16923.9
BF	304U_A	0.1447471	0.0000000	3861.9	0	16715.9	0	2481.69	0	1033.33	0
BF	305F_A	0.1279776	0.0000000	3414.48	0	14779.3	0	2194.18	0	913.612	0
TOTAL		1.0000	1.0000								

LEVEL		RF		WEST				EAST			
TORSIONAL SHEAR		RELATIVE RIGIDITY		(+)		(-)		(+)		(-)	
				TORSION = -7118126		TORSION = -4057852		TORSION = -591748		TORSION = 1395606	
		SHEAR WALL		X-DIRECTION	Y-DIRECTION	X	Y	X	Y	X	Y
BF	304F	0.0000000	0.0005187	0	-3692.24	0	-2104.84	0	-306.945	0	723.914
BF	304G	0.0016655	0.0000000	-11855.1	0	-6758.27	0	-985.544	0	2324.35	0
BF	304H	0.0000000	0.0027119	0	-19303.3	0	-11004.3	0	-1604.73	0	3784.68
BF	304J	0.0006618	0.0000000	-4711.04	0	-2685.64	0	-391.641	0	923.665	0
BF	304K	0.0005276	0.0001722	-3755.39	-1225.78	-2140.85	-698.781	-312.195	-101.902	736.296	240.33
BF	304R	0.0000000	0.0005574	0	-3967.8	0	-2261.94	0	-329.853	0	777.942
BF	304S	0.0000000	0.0008711	0	-6200.64	0	-3534.82	0	-515.475	0	1215.72
BF	304T	0.0000000	0.0021874	0	-15570.1	0	-8876.07	0	-1294.38	0	3052.72
BF	304U	0.0000920	0.0000000	-654.941	0	-373.364	0	-54.4469	0	128.41	0
BF	305F	0.0018788	0.0000000	-13373.5	0	-7623.86	0	-1111.77	0	2622.06	0
BF	304T_A	0.0000000	0.0016065	0	-11435.2	0	-6518.91	0	-950.639	0	2242.03
BF	304U_A	0.0000493	0.0000000	-351.007	0	-200.1	0	-29.1801	0	68.8198	0
BF	305F_A	0.0008348	0.0000000	-5942.09	0	-3387.42	0	-493.981	0	1165.03	0

LEVEL		RF		WEST				EAST			
TOTAL SHEAR				(+)		(-)		(+)		(-)	
				Vx	Vy	Vx	Vy	Vx	Vy	Vx	Vy
SHEAR WALL		X	Y	X	Y	X	Y	X	Y	X	Y
BF	304F	0	12179.8	0	2914.58	0	5646.85	0	4740.52		
BF	304G	-9414.38	0	3806.18	0	582.881	0	2977.41	0		
BF	304H	0	34823.2	0	12484.9	0	11368.9	0	6207.24		
BF	304J	-3242.45	0	3671.08	0	552.093	0	1316.62	0		
BF	304K	-2584.7	3467.66	2926.39	912.662	440.099	1512.37	1049.54	1203.03		
BF	304R	0	11224.7	0	2954.26	0	4895.49	0	3894.17		
BF	304S	0	28768.6	0	5687.85	0	14714	0	13313.9		
BF	304T	0	51362	0	12290.7	0	23812.6	0	19990.7		
BF	304U	6550.94	0	30816.8	0	4576.12	0	2056.48	0		
BF	305F	-6255.43	0	23186.1	0	3462.36	0	4526.63	0		
BF	304T_A	0	37722.2	0	9026.74	0	17488.9	0	14681.9		
BF	304U_A	3510.89	0	16515.8	0	2452.51	0	1102.15	0		
BF	305F_A	-2527.61	0	11391.9	0	1700.2	0	2078.64	0		
TOTAL											

APPENDIX 9

LEVEL	PH	LOAD APP: 8					
WIND CASE	INTERNAL PRESSURE	WINDWARD	LEEWARD	NET	SIDE 1	SIDE 2	NET
WEST	(+)	12059	-23060	35118	-41298	-34030	7268
	(-)	19620	-9742	29362	-17448	-14377	3071
EAST	(+)	14112	-8413	22525	-26538	-21868	4670
	(-)	22760	-3503	26264	-11050	-9105	1945
SOUTH	(+)	28581	-25606	54187	-9852	-17352	-7500
	(-)	46717	-10662	57378	-4102	-7225	-3123
NORTH	(+)	24097	-31075	55171	-9852	-17352	-7500
	(-)	39041	-12939	51980	-4102	-7225	-3123

LEVEL	PH	LOAD APP: 8					
WIND CASE	INTERNAL PRESSURE	WINDWARD		RESULTANT COORDINATE:		ECCENTRICITY	
		Fs X	Fs Y	X	Y	Ex	Ey
WEST	(+)	2574528	0	213.503	0	13.4813	0
	(-)	4189428	0	213.533	0	13.5119	0
EAST	(+)	3049129	0	216.064	0	16.0429	0
	(-)	4919534	0	216.144	0	16.1223	0
SOUTH	(+)	0	6695318	0	234.261	0	20.9239
	(-)	0	10939299	0	234.163	0	20.8265
NORTH	(+)	0	5308526	0	220.302	0	6.96497
	(-)	0	8607570	0	220.476	0	7.13953

C.O.R.

X = 2400.2589

Y = 2560.04

LEEWARD		RESULTANT COORDINATE:		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
-4987223	0	216.273	0	16.2518	0
-2107035	0	216.273	0	16.2518	0
-1796927	0	213.582	0	13.5607	0
-748205	0	213.582	0	13.5607	0
0	-5652729	0	220.758	0	7.42099
0	-2353684	0	220.758	0	7.42099
0	-7271824	0	234.01	0	20.673
0	-3027843	0	234.01	0	20.673

SIDE 1		RESULTANT COORDINATE:		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	-9664028	0	234.01	0	20.673
0	-4082923	0	234.01	0	20.673
0	-6210193	0	234.01	0	20.673
0	-2585801	0	234.01	0	20.673
-2104111	0	213.582	0	13.5607	0
-876110	0	213.582	0	13.5607	0
-2104111	0	213.582	0	13.5607	0
-876110	0	213.582	0	13.5607	0

SIDE 2		RESULTANT COORDINATE:		ECCENTRICITY	
Fs X	Fs Y	X	Y	Ex	Ey
0	-7512301	0	220.758	0	7.42099
0	-3173847	0	220.758	0	7.42099
0	-4827474	0	220.758	0	7.42099
0	-2010064	0	220.758	0	7.42099
-3752701	0	216.273	0	16.2518	0
-1562550	0	216.273	0	16.2518	0
-3752701	0	216.273	0	16.2518	0
-1562550	0	216.273	0	16.2518	0

LEVEL	PH	[=] FT LB										
WIND CASE	INTERNAL PRESSURE	WINDWARD		LEEWARD		SIDE 1		SIDE 2		FEX	FEY	TORSION
		F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)	F(Ex)	F(Ey)			
WEST	(+)	162563.99	0	-374763	0	0	-853745	0	-252533.56	537327.21	601211	-63884.1
	(-)	265097	0	-158332	0	0	-360696	0	-106692.05	423429.37	254004	169426
EAST	(+)	226400	0	-114090	0	0	-548624	0	-162280.38	-340490	386344	-726834
	(-)	366951	0	-47504.9	0	0	-228436	0	-67570.331	-414456	160866	-575321
SOUTH	(+)	0	598019	0	-190022	-133594	0	-281995	0	148402	-788041	936443
	(-)	0	972943	0	-79121.5	-55625.8	0	-117417	0	61791	-1052064	1113856
NORTH	(+)	0	167832	0	-642411	-133594	0	-281995	0	148402	810244	-661842
	(-)	0	278733	0	-267487	-55625.8	0	-117417	0	61791	546220	-484429



Wall Section Force Summary

RAM FRAME v11.0
Jonathan Williams
Database: FHCRC_EXISTING

01/11/08 20:00:47

CRITERIA:

Wall Mesh
 Max Distance between Nodes (ft): 8.00
 P-Delta: Yes Scale Factor: 1.00
 Diaphragm: Rigid
 Ground Level: Floor_E

LOAD CASES:

D DeadLoad RAMUSER
 Lp PosLiveLoad RAMUSER
 Ln NegLiveLoad RAMUSER
 E1 Seismic EQ_IBC03_X_NoE_F
 E2 Seismic EQ_IBC03_Y_NoE_F
 W1 Wind Wind_IBC03_1_X
 W2 Wind Wind_IBC03_1_Y
 W3 Wind Wind_IBC03_2_X+E
 W4 Wind Wind_IBC03_2_X-E
 W5 Wind Wind_IBC03_2_Y+E
 W6 Wind Wind_IBC03_2_Y-E
 W7 Wind Wind_IBC03_3_X+Y
 W8 Wind Wind_IBC03_3_X-Y
 W9 Wind Wind_IBC03_4_X+Y_CW
 W10 Wind Wind_IBC03_4_X+Y_CCW
 W11 Wind Wind_IBC03_4_X-Y_CW
 W12 Wind Wind_IBC03_4_X-Y_CCW
 E7 Seismic Eccent EQ_IBC03_X_+E_F
 E8 Seismic Eccent EQ_IBC03_X_-E_F
 E9 Seismic Eccent EQ_IBC03_Y_+E_F
 E10 Seismic Eccent EQ_IBC03_Y_-E_F

Level: Floor_E

Section: 12

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	683.10	-24.76	-0.00	16.25	-0.00	-0.00
Lp	235.55	-8.32	-0.00	5.47	-0.00	-0.00
Ln	-1.28	-0.06	-0.00	0.04	-0.00	0.00
E1	1827.12	-267.36	-0.00	183.08	-0.00	-0.00
E2	-2583.10	572.69	0.00	-391.04	-0.00	0.00
W1	339.53	-58.23	-0.00	39.83	-0.00	-0.00
W2	-365.32	88.44	0.00	-60.36	0.00	0.00
W3	-9.34	16.66	0.00	-11.31	0.00	-0.00



Wall Section Force Summary

W4	234.91	-37.54	-0.00	25.69	-0.00	-0.00
W5	-204.31	47.18	-0.00	-32.21	-0.00	0.00
W6	-316.37	71.59	0.00	-48.87	0.00	0.00
W7	-19.34	22.66	-0.00	-15.40	-0.00	0.00
W8	528.64	-110.00	-0.00	75.14	-0.00	-0.00
W9	-148.13	46.47	-0.00	-31.67	-0.00	0.00
W10	119.11	-12.49	-0.00	8.58	-0.00	-0.00
W11	262.86	-53.02	-0.00	36.23	-0.00	-0.00
W12	530.10	-111.98	-0.00	76.48	-0.00	-0.00
E7	1013.77	-138.57	-0.00	94.94	-0.00	-0.00
E8	1434.64	-219.71	-0.00	150.38	-0.00	-0.00
E9	-1587.19	356.49	-0.00	-243.40	-0.00	0.00
E10	-1874.27	410.93	0.00	-280.60	0.00	0.00

Section: SC15

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1210.56	-1628.08	0.00	-69.95	0.00	-0.00
Lp	272.51	-859.38	0.00	-36.91	0.00	-0.00
Ln	-0.00	85.07	0.00	3.62	0.00	-0.00
E1	-0.00	35790.59	0.00	1566.64	0.00	-0.00
E2	-0.00	40740.36	-0.01	1791.49	-0.00	-0.00
W1	-0.00	6671.84	0.00	292.83	0.00	-0.00
W2	-0.00	5354.90	-0.00	236.58	-0.00	-0.00
W3	-0.00	4039.48	-0.00	181.17	-0.00	-0.00
W4	-0.00	4999.79	0.00	219.79	0.00	-0.00
W5	-0.00	4167.79	-0.00	182.94	-0.00	-0.00
W6	-0.00	3716.83	-0.00	164.75	-0.00	-0.00
W7	-0.00	9020.05	-0.00	397.06	-0.00	-0.00
W8	-0.00	987.71	0.00	42.18	0.00	-0.00
W9	-0.00	6235.81	-0.00	276.49	-0.00	-0.00
W10	-0.00	7294.26	0.00	319.10	0.00	-0.00
W11	0.00	211.56	0.00	10.33	0.00	-0.00
W12	-0.00	1270.00	0.00	52.94	0.00	0.00
E7	-0.00	23052.98	0.00	1011.69	0.00	-0.00
E8	-0.00	24907.86	0.00	1087.67	0.00	-0.00
E9	-0.00	27935.54	-0.01	1226.54	-0.00	-0.00
E10	-0.00	26658.19	-0.01	1174.13	-0.00	-0.00

Section: SC16

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1202.00	-1728.46	-0.00	-74.25	-0.00	0.00
Lp	299.12	-830.46	0.00	-35.75	0.00	0.00



Wall Section Force Summary

Ln	-0.00	68.81	0.00	2.94	-0.00	0.00
E1	0.00	24912.55	0.00	1079.00	0.00	0.00
E2	-0.00	34335.22	-0.01	1491.51	-0.00	0.00
W1	0.00	4810.83	0.00	209.28	0.00	0.00
W2	-0.00	3911.65	-0.00	169.88	-0.00	0.00
W3	-0.00	-2288.40	-0.00	-101.48	-0.00	-0.00
W4	0.00	3167.49	0.00	137.40	0.00	0.00
W5	-0.00	4487.00	-0.00	195.96	-0.00	0.00
W6	-0.00	1998.38	-0.00	87.06	-0.00	0.00
W7	-0.00	6541.86	-0.00	284.37	-0.00	0.00
W8	0.00	674.38	0.00	29.55	0.00	0.00
W9	-0.00	1927.20	-0.00	82.86	-0.00	-0.00
W10	-0.00	7885.59	0.00	343.70	0.00	0.00
W11	0.00	-2473.41	0.00	-108.26	0.00	-0.00
W12	0.00	3484.98	0.00	152.58	0.00	0.00
E7	-0.00	12123.97	0.00	523.74	0.00	0.00
E8	0.00	21259.86	0.00	922.16	0.00	0.00
E9	-0.00	26106.07	-0.01	1134.51	-0.00	0.00
E10	-0.00	19904.51	-0.01	864.17	-0.00	0.00

Section: SC17

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	669.66	-8.39	0.00	3.91	0.00	-0.00
Lp	197.16	-1.01	0.00	0.34	0.00	-0.00
Ln	1.69	-0.05	-0.00	0.03	-0.00	-0.00
E1	-1670.63	-252.25	-0.00	157.08	-0.00	0.00
E2	2627.41	650.51	-0.00	-404.86	-0.00	-0.00
W1	-313.09	-56.50	-0.00	35.18	-0.00	0.00
W2	378.58	101.98	-0.00	-63.46	-0.00	-0.00
W3	102.42	38.67	-0.00	-24.06	-0.00	-0.00
W4	-209.45	-34.17	-0.00	21.28	-0.00	0.00
W5	195.17	50.89	-0.00	-31.67	-0.00	-0.00
W6	338.09	83.69	-0.00	-52.09	-0.00	-0.00
W7	49.11	34.11	-0.00	-21.22	-0.00	-0.00
W8	-518.75	-118.86	-0.00	73.98	-0.00	0.00
W9	207.38	65.20	-0.00	-40.57	-0.00	-0.00
W10	-133.71	-14.04	-0.00	8.75	-0.00	0.00
W11	-218.52	-49.53	0.00	30.83	0.00	0.00
W12	-559.61	-128.76	-0.00	80.14	-0.00	0.00
E7	-851.02	-114.33	-0.00	71.21	-0.00	0.00
E8	-1387.68	-223.70	-0.00	139.29	-0.00	0.00
E9	1577.49	399.16	-0.00	-248.42	-0.00	-0.00
E10	1943.34	472.55	-0.00	-294.11	-0.00	-0.00



Wall Section Force Summary

Section: SC18

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	753.32	3.20	0.00	-0.63	-0.00	0.00
Lp	260.03	-2.56	0.00	1.90	0.00	0.00
Ln	-1.69	-0.05	-0.00	0.03	-0.00	-0.00
E1	1670.63	-252.04	-0.00	157.21	-0.00	0.00
E2	-2627.41	652.17	-0.00	-406.55	-0.00	-0.00
W1	313.09	-56.53	-0.00	35.25	-0.00	0.00
W2	-378.58	102.28	-0.00	-63.75	-0.00	-0.00
W3	-102.42	38.84	0.00	-24.20	0.00	-0.00
W4	209.45	-34.16	-0.00	21.31	-0.00	0.00
W5	-195.17	51.03	-0.00	-31.81	-0.00	-0.00
W6	-338.09	83.91	-0.00	-52.30	-0.00	-0.00
W7	-49.11	34.31	-0.00	-21.38	-0.00	-0.00
W8	518.75	-119.11	-0.00	74.26	-0.00	0.00
W9	-207.38	65.44	-0.00	-40.78	-0.00	-0.00
W10	133.71	-13.97	-0.00	8.72	-0.00	0.00
W11	218.52	-49.63	0.00	30.94	0.00	0.00
W12	559.61	-129.04	-0.00	80.44	-0.00	0.00
E7	851.02	-114.11	-0.00	71.19	-0.00	0.00
E8	1387.68	-223.63	-0.00	139.47	-0.00	0.00
E9	-1577.49	400.22	-0.00	-249.48	-0.00	-0.00
E10	-1943.34	473.71	-0.00	-295.30	-0.00	-0.00

Section: SC19

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	596.29	21.91	0.00	-12.72	0.00	0.00
Lp	175.02	5.55	0.00	-3.17	0.00	0.00
Ln	1.28	-0.07	-0.00	0.04	-0.00	0.00
E1	-1827.12	-298.90	-0.00	183.01	-0.00	-0.00
E2	2583.10	637.88	-0.00	-389.79	-0.00	0.00
W1	-339.53	-64.99	-0.00	39.76	-0.00	-0.00
W2	365.32	98.44	-0.00	-60.14	-0.00	0.00
W3	9.34	18.44	-0.00	-11.22	-0.00	-0.00
W4	-234.91	-41.93	-0.00	25.66	-0.00	-0.00
W5	204.31	52.53	-0.00	-32.10	-0.00	0.00
W6	316.37	79.73	-0.00	-48.71	-0.00	0.00
W7	19.34	25.09	-0.00	-15.28	-0.00	-0.00
W8	-528.64	-122.58	-0.00	74.92	-0.00	-0.00
W9	148.13	51.65	-0.00	-31.52	-0.00	0.00
W10	-119.11	-14.02	-0.00	8.60	-0.00	-0.00
W11	-262.86	-59.10	0.00	36.13	-0.00	-0.00



Wall Section Force Summary

W12	-530.10	-124.77	-0.00	76.26	-0.00	-0.00
E7	-1013.77	-155.03	-0.00	94.96	-0.00	-0.00
E8	-1434.64	-245.51	-0.00	150.28	-0.00	-0.00
E9	1587.19	397.03	-0.00	-242.61	-0.00	0.00
E10	1874.27	457.75	-0.00	-279.73	-0.00	0.00

Section: SC21

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1066.11	-396.54	0.00	-46.56	0.00	-0.00
Lp	276.37	-179.67	0.00	-20.98	0.00	-0.00
Ln	0.00	11.34	0.00	1.34	0.00	-0.00
E1	-0.00	2879.19	0.00	335.11	0.00	-0.00
E2	0.00	5877.06	-0.01	687.82	-0.00	-0.00
W1	-0.00	629.86	0.00	73.84	0.00	-0.00
W2	0.00	494.80	-0.00	57.09	-0.00	-0.00
W3	0.00	-1996.69	-0.00	-239.90	-0.00	0.00
W4	-0.00	259.46	0.00	29.80	0.00	-0.00
W5	0.00	1065.97	-0.00	126.24	-0.00	-0.00
W6	0.00	42.76	-0.00	3.95	-0.00	-0.00
W7	0.00	843.49	-0.00	98.20	-0.00	-0.00
W8	-0.00	101.29	0.00	12.57	0.00	-0.00
W9	0.00	-597.15	-0.00	-73.35	-0.00	-0.00
W10	-0.00	1862.38	0.00	220.64	0.00	-0.00
W11	-0.00	-1153.79	0.00	-137.57	0.00	0.00
W12	-0.00	1305.74	0.00	156.42	0.00	-0.00
E7	-0.00	103.76	0.00	6.91	0.00	-0.00
E8	-0.00	3754.47	0.00	442.15	0.00	-0.00
E9	0.00	5172.07	-0.00	607.98	-0.00	-0.00
E10	0.00	2703.43	-0.00	313.74	-0.00	-0.00

Section: SC22

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1183.93	-219.94	0.00	-16.28	0.00	0.00
Lp	504.83	-120.32	0.00	-8.83	0.00	0.00
Ln	-76.46	22.65	-0.00	1.61	-0.00	-0.00
E1	5.81	22632.93	0.00	1632.91	0.00	-0.00
E2	-42.21	-3262.99	-0.00	-240.59	-0.00	-0.00
W1	0.82	4295.10	0.00	310.67	0.00	-0.00
W2	-7.23	-423.36	-0.00	-31.25	-0.00	-0.00
W3	18.39	3554.03	0.00	257.82	0.00	-0.00
W4	18.41	3271.82	0.00	236.73	0.00	-0.00
W5	-30.23	-433.29	-0.00	-32.06	-0.00	-0.00



Wall Section Force Summary

W6	-31.30	-311.36	-0.00	-22.93	-0.00	-0.00
W7	-4.80	2903.81	-0.00	209.57	-0.00	-0.00
W8	6.04	3538.85	0.00	256.44	0.00	0.00
W9	-4.01	2329.41	-0.00	168.51	-0.00	-0.00
W10	-3.19	2026.30	-0.00	145.84	-0.00	0.00
W11	4.12	2805.69	0.00	203.67	0.00	-0.00
W12	4.94	2502.58	0.00	181.00	0.00	0.00
E7	3.99	15358.42	0.00	1108.59	0.00	-0.00
E8	3.80	14970.61	0.00	1079.58	0.00	0.00
E9	-28.23	-2313.60	-0.00	-170.73	-0.00	-0.00
E10	-28.32	-2058.94	-0.00	-151.67	-0.00	-0.00

Section: SC23

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1183.93	-219.94	0.00	-16.28	0.00	0.00
Lp	504.83	-120.32	0.00	-8.83	0.00	0.00
Ln	-76.46	22.65	-0.00	1.61	-0.00	-0.00
E1	5.81	22632.93	0.00	1632.91	0.00	-0.00
E2	-42.21	-3262.99	-0.00	-240.59	-0.00	-0.00
W1	0.82	4295.10	0.00	310.67	0.00	-0.00
W2	-7.23	-423.36	-0.00	-31.25	-0.00	-0.00
W3	18.39	3554.03	0.00	257.82	0.00	-0.00
W4	18.41	3271.82	0.00	236.73	0.00	-0.00
W5	-30.23	-433.29	-0.00	-32.06	-0.00	-0.00
W6	-31.30	-311.36	-0.00	-22.93	-0.00	-0.00
W7	-4.80	2903.81	-0.00	209.57	-0.00	-0.00
W8	6.04	3538.85	0.00	256.44	0.00	0.00
W9	-4.01	2329.41	-0.00	168.51	-0.00	-0.00
W10	-3.19	2026.30	-0.00	145.84	-0.00	0.00
W11	4.12	2805.69	0.00	203.67	0.00	-0.00
W12	4.94	2502.58	0.00	181.00	0.00	0.00
E7	3.99	15358.42	0.00	1108.59	0.00	-0.00
E8	3.80	14970.61	0.00	1079.58	0.00	0.00
E9	-28.23	-2313.60	-0.00	-170.73	-0.00	-0.00
E10	-28.32	-2058.94	-0.00	-151.67	-0.00	-0.00

Section: SC24

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1616.83	-71.14	0.00	-37.29	0.00	0.00
Lp	634.03	-30.19	0.00	-15.27	0.00	0.00
Ln	-3.15	0.86	0.00	0.38	0.00	-0.00
E1	687.26	-588.02	0.00	-268.96	0.00	0.00



Wall Section Force Summary

E2	-2832.17	1781.52	-0.00	807.34	-0.00	-0.00
W1	119.97	-116.13	0.00	-53.31	0.00	0.00
W2	-358.50	234.60	-0.00	106.57	-0.00	-0.00
W3	258.80	-177.47	0.00	-80.38	0.00	0.00
W4	82.85	-94.47	0.00	-43.27	0.00	0.00
W5	-291.58	199.79	-0.00	90.60	-0.00	-0.00
W6	-208.97	162.17	-0.00	73.78	-0.00	-0.00
W7	-178.90	88.85	0.00	39.95	0.00	-0.00
W8	358.85	-263.05	0.00	-119.91	0.00	0.00
W9	-37.21	21.41	0.00	9.74	0.00	-0.00
W10	-231.13	111.87	0.00	50.19	0.00	-0.00
W11	366.10	-242.52	0.00	-110.16	0.00	0.00
W12	172.18	-152.06	0.00	-69.71	0.00	0.00
E7	611.27	-462.97	0.00	-211.02	0.00	0.00
E8	309.68	-325.00	0.00	-149.40	0.00	0.00
E9	-2002.08	1240.34	-0.00	561.78	-0.00	-0.00
E10	-1793.14	1146.96	-0.00	520.08	-0.00	-0.00

Section: SC25

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1204.64	-6.32	0.00	2.94	0.00	-0.00
Lp	447.26	-9.03	0.00	-1.97	0.00	-0.00
Ln	2.31	0.86	0.00	0.38	0.00	-0.00
E1	-687.26	-588.49	0.00	-269.27	0.00	0.00
E2	2832.17	1783.24	-0.00	808.71	0.00	-0.00
W1	-119.97	-116.22	0.00	-53.36	0.00	0.00
W2	358.50	234.81	-0.00	106.74	0.00	-0.00
W3	-258.80	-177.66	0.00	-80.52	0.00	0.00
W4	-82.85	-94.56	0.00	-43.32	0.00	0.00
W5	291.58	199.99	-0.00	90.75	-0.00	-0.00
W6	208.97	162.33	0.00	73.88	0.00	-0.00
W7	178.90	88.94	0.00	40.04	0.00	-0.00
W8	-358.85	-263.27	0.00	-120.07	0.00	0.00
W9	37.21	21.42	0.00	9.75	0.00	-0.00
W10	231.13	111.99	0.00	50.30	0.00	-0.00
W11	-366.10	-242.74	0.00	-110.33	0.00	0.00
W12	-172.18	-152.16	0.00	-69.78	0.00	0.00
E7	-611.27	-463.38	0.00	-211.30	0.00	0.00
E8	-309.68	-325.22	0.00	-149.53	0.00	0.00
E9	2002.08	1241.56	-0.00	562.76	-0.00	-0.00
E10	1793.14	1148.05	-0.00	520.95	0.00	-0.00

Section: SC26



Wall Section Force Summary

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	645.15	9.40	0.00	-5.79	0.00	0.00
Lp	281.02	4.52	0.00	-2.79	0.00	0.00
Ln	-0.23	-0.13	0.00	0.08	0.00	-0.00
E1	401.01	125.05	0.00	-77.10	0.00	0.00
E2	-1173.26	-356.55	0.00	220.29	0.00	-0.00
W1	70.52	25.33	0.00	-15.61	0.00	0.00
W2	-137.73	-47.79	0.00	29.50	0.00	-0.00
W3	143.57	35.07	0.00	-21.67	0.00	0.00
W4	61.92	20.20	0.00	-12.45	0.00	0.00
W5	-130.03	-40.05	-0.00	24.74	-0.00	-0.00
W6	-92.55	-33.29	0.00	20.55	0.00	-0.00
W7	-50.41	-16.84	0.00	10.42	0.00	-0.00
W8	156.18	54.84	0.00	-33.83	0.00	0.00
W9	6.87	-4.52	0.00	2.79	0.00	-0.00
W10	-82.48	-20.74	0.00	12.85	0.00	-0.00
W11	161.81	49.24	0.00	-30.41	0.00	0.00
W12	72.46	33.02	0.00	-20.34	0.00	0.00
E7	344.98	96.11	0.00	-59.30	0.00	0.00
E8	192.39	71.46	0.00	-44.01	0.00	0.00
E9	-837.45	-247.25	-0.00	152.78	-0.00	-0.00
E10	-734.76	-230.55	0.00	142.42	0.00	-0.00

Section: SC27

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	678.38	-12.94	0.00	7.87	0.00	-0.00
Lp	294.56	-3.74	0.00	2.25	0.00	-0.00
Ln	2.12	-0.14	0.00	0.09	0.00	-0.00
E1	-145.56	124.37	0.00	-76.69	0.00	0.00
E2	1539.50	-357.45	0.00	220.82	0.00	-0.00
W1	-22.76	25.21	0.00	-15.53	0.00	0.00
W2	195.46	-47.93	0.00	29.59	0.00	-0.00
W3	-93.41	34.93	0.00	-21.59	0.00	0.00
W4	-10.81	20.07	0.00	-12.37	0.00	0.00
W5	154.83	-40.10	-0.00	24.77	-0.00	-0.00
W6	115.73	-33.34	0.00	20.58	0.00	-0.00
W7	129.52	-17.04	0.00	10.55	0.00	-0.00
W8	-163.67	54.85	0.00	-33.84	0.00	0.00
W9	51.50	-4.67	0.00	2.88	0.00	-0.00
W10	142.78	-20.89	0.00	12.94	0.00	-0.00
W11	-168.39	49.25	0.00	-30.41	0.00	0.00
W12	-77.11	33.03	0.00	-20.35	0.00	0.00
E7	-168.53	95.64	0.00	-59.02	0.00	0.00



Wall Section Force Summary

E8	-26.53	71.02	0.00	-43.75	0.00	0.00
E9	1081.13	-247.84	0.00	153.13	0.00	-0.00
E10	981.87	-231.16	0.00	142.77	0.00	-0.00

Section: SC28

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1001.84	22.54	-0.00	-14.02	-0.00	-0.00
Lp	469.97	8.58	-0.00	-5.36	-0.00	-0.00
Ln	-14.76	-0.41	0.00	0.25	0.00	0.00
E1	54.31	26.74	0.00	-16.48	0.00	-0.00
E2	-1915.99	-419.40	0.00	259.35	0.00	0.00
W1	10.42	8.32	0.00	-5.12	0.00	-0.00
W2	-251.85	-62.21	0.00	38.43	0.00	0.00
W3	-40.49	-19.92	0.00	12.25	0.00	0.00
W4	3.83	3.31	0.00	-2.03	0.00	-0.00
W5	-175.26	-37.85	-0.00	23.41	0.00	0.00
W6	-195.75	-48.25	0.00	29.81	0.00	0.00
W7	-181.07	-40.42	0.00	24.99	0.00	0.00
W8	196.71	52.90	0.00	-32.66	0.00	-0.00
W9	-160.11	-42.93	0.00	26.50	0.00	0.00
W10	-111.50	-17.70	0.00	10.98	0.00	0.00
W11	123.23	27.06	0.00	-16.74	0.00	-0.00
W12	171.83	52.29	0.00	-32.25	0.00	-0.00
E7	-2.83	1.26	0.00	-0.79	0.00	-0.00
E8	75.61	34.57	0.00	-21.30	0.00	-0.00
E9	-1256.78	-269.89	0.00	166.92	0.00	0.00
E10	-1310.72	-292.13	0.00	180.61	0.00	0.00

Section: SC29

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	716.91	12.15	-0.00	-7.33	-0.00	0.00
Lp	323.16	6.49	-0.00	-3.93	-0.00	0.00
Ln	-3.24	-0.23	0.00	0.14	0.00	-0.00
E1	-54.31	26.74	0.00	-16.48	0.00	-0.00
E2	1915.99	-419.10	-0.00	259.18	-0.00	0.00
W1	-10.42	8.32	0.00	-5.12	0.00	-0.00
W2	251.85	-62.17	-0.00	38.41	-0.00	0.00
W3	40.49	-19.91	0.00	12.24	0.00	0.00
W4	-3.83	3.31	0.00	-2.03	0.00	-0.00
W5	175.26	-37.82	-0.00	23.40	-0.00	0.00
W6	195.75	-48.22	0.00	29.79	-0.00	0.00
W7	181.07	-40.39	0.00	24.97	0.00	0.00



Wall Section Force Summary

W8	-196.71	52.87	0.00	-32.65	0.00	-0.00
W9	160.11	-42.90	0.00	26.48	0.00	0.00
W10	111.50	-17.69	0.00	10.98	0.00	0.00
W11	-123.23	27.05	0.00	-16.73	0.00	-0.00
W12	-171.83	52.26	0.00	-32.24	0.00	-0.00
E7	2.83	1.27	0.00	-0.80	0.00	-0.00
E8	-75.61	34.56	0.00	-21.29	0.00	-0.00
E9	1256.78	-269.70	-0.00	166.82	-0.00	0.00
E10	1310.72	-291.92	-0.00	180.50	-0.00	0.00

Section: SC30

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1005.63	26.98	-0.00	-16.72	-0.00	-0.00
Lp	478.41	9.87	-0.00	-6.14	-0.00	-0.00
Ln	-23.52	-0.61	0.00	0.37	0.00	0.00
E1	-68.19	-8.64	0.00	5.39	0.00	-0.00
E2	-1948.84	-452.05	0.00	279.38	0.00	0.00
W1	-9.81	2.19	0.00	-1.33	0.00	-0.00
W2	-262.39	-68.78	0.00	42.47	0.00	0.00
W3	-119.68	-40.02	0.00	24.68	0.00	-0.00
W4	-14.90	-2.79	0.00	1.73	0.00	-0.00
W5	-167.69	-37.98	-0.00	23.48	-0.00	0.00
W6	-216.06	-54.70	0.00	33.78	0.00	0.00
W7	-204.15	-49.94	0.00	30.86	0.00	0.00
W8	189.43	53.23	0.00	-32.85	0.00	-0.00
W9	-210.54	-57.69	0.00	35.61	0.00	0.00
W10	-95.68	-17.22	0.00	10.68	0.00	0.00
W11	84.64	19.69	0.00	-12.17	0.00	-0.00
W12	199.51	60.16	0.00	-37.11	0.00	-0.00
E7	-138.31	-33.06	0.00	20.43	0.00	-0.00
E8	46.93	21.48	0.00	-13.21	0.00	-0.00
E9	-1242.27	-284.63	-0.00	175.94	-0.00	0.00
E10	-1369.25	-321.13	0.00	198.45	0.00	0.00

Section: SC31

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	714.03	11.67	-0.00	-7.04	-0.00	0.00
Lp	325.71	6.43	-0.00	-3.89	-0.00	0.00
Ln	-7.28	-0.30	0.00	0.19	0.00	-0.00
E1	68.19	-8.63	0.00	5.38	0.00	-0.00
E2	1948.84	-451.73	0.00	279.21	-0.00	0.00
W1	9.81	2.19	0.00	-1.33	0.00	-0.00



Wall Section Force Summary

W2	262.39	-68.74	0.00	42.45	0.00	0.00
W3	119.68	-40.00	0.00	24.66	0.00	-0.00
W4	14.90	-2.79	0.00	1.73	0.00	-0.00
W5	167.69	-37.95	-0.00	23.46	-0.00	0.00
W6	216.06	-54.67	0.00	33.76	0.00	0.00
W7	204.15	-49.91	0.00	30.84	0.00	0.00
W8	-189.43	53.20	0.00	-32.83	0.00	-0.00
W9	210.54	-57.66	0.00	35.59	0.00	0.00
W10	95.68	-17.21	0.00	10.67	0.00	0.00
W11	-84.64	19.68	0.00	-12.16	0.00	-0.00
W12	-199.51	60.12	0.00	-37.09	0.00	-0.00
E7	138.31	-33.03	0.00	20.41	0.00	-0.00
E8	-46.93	21.47	0.00	-13.20	0.00	-0.00
E9	1242.27	-284.43	-0.00	175.82	-0.00	0.00
E10	1369.25	-320.90	0.00	198.33	0.00	0.00

Section: SC33

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	947.66	23.01	-0.00	-14.26	-0.00	-0.00
Lp	388.58	8.57	-0.00	-5.32	-0.00	-0.00
Ln	-5.13	-0.37	-0.00	0.23	0.00	0.00
E1	-2538.40	-524.80	0.00	325.01	0.00	0.00
E2	142.14	-29.35	-0.00	17.86	-0.00	-0.00
W1	-444.74	-100.77	0.00	62.37	0.00	0.00
W2	0.33	-11.42	-0.00	7.01	-0.00	0.00
W3	-527.40	-136.47	-0.00	84.40	-0.00	0.00
W4	-349.31	-81.65	0.00	50.53	0.00	0.00
W5	51.87	10.51	-0.00	-6.51	-0.00	-0.00
W6	-29.39	-14.15	-0.00	8.72	-0.00	0.00
W7	-333.30	-84.14	-0.00	52.03	-0.00	0.00
W8	-333.81	-67.01	0.00	41.52	0.00	0.00
W9	-347.23	-92.91	-0.00	57.44	-0.00	0.00
W10	-152.72	-33.30	-0.00	20.61	-0.00	0.00
W11	-347.61	-80.06	0.00	49.55	0.00	0.00
W12	-153.10	-20.46	0.00	12.72	0.00	0.00
E7	-1859.98	-392.64	-0.00	243.12	0.00	0.00
E8	-1541.57	-310.62	0.00	192.40	0.00	0.00
E9	203.28	7.84	-0.00	-5.05	-0.00	-0.00
E10	-12.80	-47.18	-0.00	28.99	-0.00	0.00

Section: SC34



Wall Section Force Summary

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	821.46	-3.07	-0.00	2.03	-0.00	0.00
Lp	316.98	0.08	-0.00	-0.00	-0.00	0.00
Ln	5.52	-0.36	-0.00	0.23	-0.00	0.00
E1	2862.01	-522.38	-0.00	323.60	-0.00	0.00
E2	-9.47	-27.80	-0.00	16.94	-0.00	-0.00
W1	502.67	-100.33	-0.00	62.11	-0.00	0.00
W2	20.98	-11.18	-0.00	6.87	-0.00	0.00
W3	602.51	-135.92	-0.00	84.08	-0.00	0.00
W4	385.52	-81.43	-0.00	50.40	-0.00	0.00
W5	-34.55	10.74	-0.00	-6.65	-0.00	-0.00
W6	65.83	-13.75	-0.00	8.48	-0.00	0.00
W7	392.74	-83.64	-0.00	51.74	-0.00	0.00
W8	361.27	-66.86	0.00	41.43	0.00	0.00
W9	413.57	-92.34	-0.00	57.11	-0.00	0.00
W10	175.54	-33.11	-0.00	20.50	-0.00	0.00
W11	389.96	-79.76	0.00	49.38	-0.00	0.00
W12	151.94	-20.53	0.00	12.77	0.00	0.00
E7	2107.77	-390.76	-0.00	242.03	-0.00	0.00
E8	1727.44	-309.25	-0.00	191.60	-0.00	0.00
E9	-136.97	8.69	-0.00	-5.55	-0.00	-0.00
E10	124.28	-45.94	-0.00	28.25	-0.00	0.00

Section: SC35

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	947.19	5.99	-0.00	-3.88	-0.00	-0.00
Lp	433.28	1.69	-0.00	-1.14	-0.00	-0.00
Ln	-5.36	-0.37	-0.00	0.23	0.00	0.00
E1	-2712.17	-524.29	0.00	324.70	0.00	0.00
E2	75.82	-28.57	-0.00	17.40	-0.00	-0.00
W1	-475.82	-100.67	0.00	62.31	0.00	0.00
W2	-10.42	-11.31	-0.00	6.95	-0.00	0.00
W3	-567.55	-136.33	-0.00	84.32	-0.00	0.00
W4	-369.06	-81.63	0.00	50.52	0.00	0.00
W5	43.37	10.63	-0.00	-6.59	-0.00	-0.00
W6	-47.88	-13.96	-0.00	8.61	-0.00	0.00
W7	-364.68	-83.98	-0.00	51.94	-0.00	0.00
W8	-349.05	-67.02	0.00	41.52	0.00	0.00
W9	-382.16	-92.72	-0.00	57.33	-0.00	0.00
W10	-164.86	-33.25	-0.00	20.58	-0.00	0.00
W11	-370.44	-80.00	0.00	49.52	0.00	0.00
W12	-153.14	-20.53	0.00	12.77	0.00	0.00
E7	-1992.72	-392.21	0.00	242.87	0.00	0.00



Wall Section Force Summary

E8	-1641.70	-310.36	0.00	192.24	0.00	0.00
E9	170.70	8.30	-0.00	-5.32	-0.00	-0.00
E10	-69.09	-46.59	-0.00	28.63	-0.00	0.00

Section: SC36

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	973.82	-17.94	-0.00	11.09	-0.00	0.00
Lp	435.11	-5.03	-0.00	3.12	-0.00	0.00
Ln	5.33	-0.37	-0.00	0.23	-0.00	0.00
E1	2712.17	-523.88	-0.00	324.47	-0.00	0.00
E2	-75.82	-28.56	-0.00	17.39	-0.00	-0.00
W1	475.82	-100.60	-0.00	62.27	-0.00	0.00
W2	10.42	-11.30	-0.00	6.94	-0.00	0.00
W3	567.55	-136.26	-0.00	84.28	-0.00	0.00
W4	369.06	-81.58	-0.00	50.50	-0.00	0.00
W5	-43.37	10.63	-0.00	-6.59	-0.00	-0.00
W6	47.88	-13.95	-0.00	8.60	-0.00	0.00
W7	364.68	-83.93	-0.00	51.91	-0.00	0.00
W8	349.05	-66.97	0.00	41.50	0.00	0.00
W9	382.16	-92.67	-0.00	57.30	-0.00	0.00
W10	164.86	-33.22	-0.00	20.57	-0.00	0.00
W11	370.44	-79.95	0.00	49.49	0.00	0.00
W12	153.14	-20.51	0.00	12.75	0.00	0.00
E7	1992.72	-391.91	-0.00	242.70	-0.00	0.00
E8	1641.70	-310.10	-0.00	192.10	-0.00	0.00
E9	-170.70	8.29	-0.00	-5.31	-0.00	-0.00
E10	69.09	-46.56	-0.00	28.62	-0.00	0.00

Level: Floor_2

Section: SC32

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1211.51	-149.21	-0.00	-67.66	-0.00	0.00
Lp	596.37	-50.84	-0.00	-43.79	-0.00	0.00
Ln	0.00	-171.24	-0.00	10.45	-0.00	-0.00
E1	-0.00	1171.86	-0.00	9.64	0.00	0.00
E2	0.00	33910.75	0.00	-1330.50	-0.00	0.00
W1	-0.00	160.47	-0.00	5.26	0.00	0.00
W2	0.00	4573.74	0.00	-178.34	-0.00	0.00
W3	-0.00	2098.45	-0.00	-76.73	0.00	0.00
W4	-0.00	241.89	-0.00	-8.63	0.00	0.00
W5	0.00	2919.17	-0.00	-109.10	-0.00	0.00



Wall Section Force Summary

W6	0.00	3778.52	-0.00	-139.04	-0.00	0.00
W7	0.00	3550.66	-0.00	-129.81	0.00	0.00
W8	-0.00	-3309.95	-0.00	137.70	0.00	-0.00
W9	0.00	3681.46	-0.00	-134.12	0.00	0.00
W10	0.00	1644.53	-0.00	-60.59	0.00	-0.00
W11	-0.00	-1464.00	-0.00	66.51	0.00	0.00
W12	-0.00	-3500.93	-0.00	140.04	0.00	-0.00
E7	-0.00	2421.66	-0.00	-51.76	0.00	0.00
E8	-0.00	-851.33	-0.00	64.68	0.00	0.00
E9	0.00	21598.08	0.00	-852.65	-0.00	0.00
E10	0.00	23843.70	0.00	-930.27	-0.00	0.00

Section: SC37

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1328.09	-507.08	-0.00	118.91	-0.00	0.00
Lp	636.94	-447.58	-0.00	91.66	-0.00	0.00
Ln	-0.03	97.96	0.00	-0.84	-0.00	0.00
E1	-0.00	46071.09	0.00	-1570.07	-0.00	0.01
E2	0.00	-1131.05	0.00	70.82	0.00	0.00
W1	-0.00	8045.22	0.00	-282.47	-0.00	0.00
W2	0.00	188.49	0.00	-6.87	0.00	0.00
W3	-0.00	9481.51	0.00	-378.09	-0.00	0.00
W4	-0.00	6179.63	0.00	-239.94	-0.00	0.00
W5	0.00	-667.95	0.00	44.73	0.00	-0.00
W6	-0.00	861.02	0.00	-15.13	0.00	0.00
W7	-0.00	6175.28	0.00	-217.01	-0.00	0.00
W8	-0.00	5892.55	-0.00	-206.70	-0.00	0.00
W9	-0.00	6443.03	0.00	-237.01	-0.00	0.00
W10	-0.00	2819.89	0.00	-88.50	-0.00	0.00
W11	-0.00	6230.98	0.00	-229.28	-0.00	0.00
W12	-0.00	2607.84	-0.00	-80.78	-0.00	0.00
E7	-0.00	33816.11	0.00	-1167.97	-0.00	0.00
E8	-0.00	27921.02	0.00	-935.99	-0.00	0.00
E9	0.00	-2777.18	0.00	124.41	0.00	-0.00
E10	0.00	1261.53	0.00	-29.52	0.00	0.00

Section: SC38

LdC	P kip	Mmajor kip-ft	Mminor kip-ft	Vmajor kip	Vminor kip	Torsion kip-ft
D	1118.72	273.25	-0.00	-65.13	-0.00	-0.00
Lp	466.44	-109.35	-0.00	-38.04	-0.00	-0.00
Ln	0.39	97.45	0.00	-0.93	0.00	0.00
E1	323.61	45804.76	0.00	-1578.04	-0.00	0.01



Wall Section Force Summary

RAM FRAME v11.0
Jonathan Williams
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E2	132.68	-1124.06	0.00	65.72	0.00	0.00
W1	57.93	7998.53	0.00	-283.56	-0.00	0.00
W2	21.32	187.41	0.00	-7.53	0.00	0.00
W3	75.12	9425.76	0.00	-378.33	0.00	0.00
W4	36.21	6143.43	0.00	-239.92	-0.00	0.00
W5	17.32	-663.73	0.00	43.58	0.00	-0.00
W6	36.44	856.22	0.00	-16.54	0.00	0.00
W7	59.44	6139.45	0.00	-218.32	0.00	0.00
W8	27.46	5858.34	-0.00	-207.03	-0.00	0.00
W9	66.34	6405.44	0.00	-238.19	0.00	0.00
W10	22.82	2803.74	0.00	-89.29	-0.00	0.00
W11	42.35	6194.61	0.00	-229.72	-0.00	0.00
W12	-1.16	2592.90	-0.00	-80.82	-0.00	0.00
E7	247.79	33620.32	0.00	-1173.69	-0.00	0.00
E8	185.87	27759.91	0.00	-940.95	-0.00	0.00
E9	66.31	-2760.65	0.00	121.36	0.00	-0.00
E10	111.48	1254.37	0.00	-33.30	0.00	0.00



Drift

CRITERIA:

Rigid End Zones: Ignore Effects
 Member Force Output: At Face of Joint
 P-Delta: Yes Scale Factor: 1.00
 Diaphragm: Rigid
 Ground Level: Floor_E

LOAD CASE DEFINITIONS:

E3	SeismicDrift	EQ_IBC03_X_+E_Drft
E4	SeismicDrift	EQ_IBC03_X_-E_Drft
E5	SeismicDrift	EQ_IBC03_Y_+E_Drft
E6	SeismicDrift	EQ_IBC03_Y_-E_Drft

RESULTS:

Location (ft): (222.995, 305.413)

Story	LdC	Displacement		Story Drift		Drift Ratio	
		X in	Y in	X in	Y in	X	Y
PentHous_Roof	E3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E4	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E5	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PH	E3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E4	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E5	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
Roof	E3	4.2811	-0.4684	0.7735	-0.1219	0.0053	0.0008
	E4	3.1917	-0.3292	0.6085	-0.0953	0.0041	0.0006
	E5	-0.3942	2.7632	-0.0567	0.3250	0.0004	0.0022
	E6	0.4409	2.6553	0.0733	0.3052	0.0005	0.0021
Mech	E3	3.5076	-0.3465	0.5352	-0.0585	0.0036	0.0004
	E4	2.5832	-0.2339	0.3970	-0.0334	0.0027	0.0002
	E5	-0.3375	2.4383	-0.0877	0.4382	0.0006	0.0030
	E6	0.3676	2.3502	0.0249	0.4158	0.0002	0.0028
Floor_5	E3	2.9724	-0.2879	0.6006	-0.0548	0.0041	0.0004
	E4	2.1862	-0.2005	0.4471	-0.0356	0.0030	0.0002
	E5	-0.2498	2.0001	-0.0607	0.3994	0.0004	0.0027
	E6	0.3427	1.9343	0.0560	0.3849	0.0004	0.0026



Drift

Story	LdC	Displacement		Story Drift		Drift Ratio	
Floor_4	E3	2.3718	-0.2331	0.5879	-0.0532	0.0040	0.0004
	E4	1.7391	-0.1649	0.4353	-0.0341	0.0030	0.0002
	E5	-0.1891	1.6006	-0.0573	0.3926	0.0004	0.0027
	E6	0.2867	1.5494	0.0583	0.3782	0.0004	0.0026
Floor_3	E3	1.7838	-0.1799	0.5445	-0.0491	0.0037	0.0003
	E4	1.3039	-0.1308	0.4003	-0.0311	0.0027	0.0002
	E5	-0.1318	1.2080	-0.0504	0.3677	0.0003	0.0025
	E6	0.2285	1.1712	0.0583	0.3542	0.0004	0.0024
Floor_2	E3	1.2394	-0.1308	0.4723	-0.0451	0.0032	0.0003
	E4	0.9035	-0.0996	0.3445	-0.0284	0.0023	0.0002
	E5	-0.0815	0.8403	-0.0479	0.3299	0.0003	0.0022
	E6	0.1702	0.8170	0.0481	0.3174	0.0003	0.0022
Floor_1	E3	0.7671	-0.0858	0.4270	-0.0460	0.0029	0.0003
	E4	0.5590	-0.0713	0.3132	-0.0329	0.0021	0.0002
	E5	-0.0336	0.5104	-0.0402	0.2858	0.0003	0.0019
	E6	0.1220	0.4995	0.0451	0.2760	0.0003	0.0019
Floor_D	E3	0.3401	-0.0397	0.3401	-0.0397	0.0023	0.0003
	E4	0.2458	-0.0384	0.2458	-0.0384	0.0017	0.0003
	E5	0.0066	0.2246	0.0066	0.2246	0.0000	0.0015
	E6	0.0769	0.2236	0.0769	0.2236	0.0005	0.0015
Floor_E	E3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E4	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E5	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	E6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000



Drift

Story	LdC	Displacement		Story Drift		Drift Ratio	
	W4	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W5	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W7	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W8	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W9	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W11	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W12	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
Roof	W1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W4	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W5	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W7	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W8	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W9	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W11	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W12	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
Mech	W1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W4	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W5	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W7	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W8	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W9	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W11	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	W12	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
Floor_5	W1	0.5448	0.0459	0.1094	0.0107	0.0007	0.0001
	W2	-0.1255	0.5586	-0.0215	0.1074	0.0001	0.0007
	W3	0.0114	0.3530	0.0047	0.0654	0.0000	0.0004
	W4	0.3908	0.0538	0.0814	0.0093	0.0006	0.0001
	W5	0.0007	0.3363	-0.0005	0.0679	0.0000	0.0005
	W6	-0.1755	0.4746	-0.0368	0.0944	0.0003	0.0006
	W7	0.3145	0.4534	0.0659	0.0886	0.0004	0.0006
	W8	0.5027	-0.3845	0.0982	-0.0726	0.0007	0.0005
	W9	0.0275	0.5041	0.0070	0.0974	0.0000	0.0007



Drift

Story	LdC	Displacement		Story Drift		Drift Ratio	
	W10	0.4442	0.1760	0.0917	0.0355	0.0006	0.0002
	W11	0.1687	-0.1243	0.0313	-0.0235	0.0002	0.0002
	W12	0.5854	-0.4524	0.1160	-0.0854	0.0008	0.0006
Floor_4	W1	0.4355	0.0353	0.1077	0.0103	0.0007	0.0001
	W2	-0.1040	0.4511	-0.0211	0.1050	0.0001	0.0007
	W3	0.0066	0.2876	0.0045	0.0647	0.0000	0.0004
	W4	0.3094	0.0445	0.0793	0.0096	0.0005	0.0001
	W5	0.0012	0.2684	0.0012	0.0651	0.0000	0.0004
	W6	-0.1387	0.3803	-0.0339	0.0908	0.0002	0.0006
	W7	0.2486	0.3648	0.0649	0.0865	0.0004	0.0006
	W8	0.4046	-0.3119	0.0966	-0.0711	0.0007	0.0005
	W9	0.0204	0.4067	0.0075	0.0952	0.0001	0.0006
	W10	0.3525	0.1405	0.0899	0.0345	0.0006	0.0002
	W11	0.1374	-0.1008	0.0312	-0.0230	0.0002	0.0002
	W12	0.4695	-0.3670	0.1136	-0.0836	0.0008	0.0006
Floor_3	W1	0.3278	0.0250	0.1022	0.0089	0.0007	0.0001
	W2	-0.0829	0.3461	-0.0202	0.0988	0.0001	0.0007
	W3	0.0021	0.2230	0.0030	0.0618	0.0000	0.0004
	W4	0.2301	0.0349	0.0736	0.0096	0.0005	0.0001
	W5	0.0001	0.2034	0.0024	0.0602	0.0000	0.0004
	W6	-0.1048	0.2894	-0.0305	0.0844	0.0002	0.0006
	W7	0.1837	0.2783	0.0615	0.0808	0.0004	0.0005
	W8	0.3080	-0.2408	0.0918	-0.0675	0.0006	0.0005
	W9	0.0130	0.3115	0.0073	0.0893	0.0000	0.0006
	W10	0.2626	0.1059	0.0849	0.0319	0.0006	0.0002
	W11	0.1062	-0.0778	0.0300	-0.0219	0.0002	0.0001
	W12	0.3558	-0.2834	0.1077	-0.0793	0.0007	0.0005
Floor_2	W1	0.2256	0.0161	0.0914	0.0064	0.0006	0.0000
	W2	-0.0627	0.2473	-0.0180	0.0887	0.0001	0.0006
	W3	-0.0008	0.1611	0.0007	0.0547	0.0000	0.0004
	W4	0.1565	0.0253	0.0642	0.0084	0.0004	0.0001
	W5	-0.0023	0.1432	0.0033	0.0536	0.0000	0.0004
	W6	-0.0743	0.2050	-0.0260	0.0749	0.0002	0.0005
	W7	0.1222	0.1976	0.0551	0.0713	0.0004	0.0005
	W8	0.2162	-0.1734	0.0821	-0.0617	0.0006	0.0004
	W9	0.0057	0.2223	0.0065	0.0788	0.0000	0.0005
	W10	0.1777	0.0741	0.0761	0.0282	0.0005	0.0002
	W11	0.0761	-0.0559	0.0267	-0.0210	0.0002	0.0001
	W12	0.2482	-0.2041	0.0964	-0.0716	0.0007	0.0005
Floor_1	W1	0.1342	0.0097	0.0768	0.0069	0.0005	0.0000
	W2	-0.0447	0.1586	-0.0164	0.0795	0.0001	0.0005

